Redesign of Salisbury Estates

Major Qualifying Project

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Presented to:
Professor Leonard Albano and the Worcester Polytechnic Institute Civil & Environmental Engineering Department in partial fulfillment of the requirements of the Degree of Bachelor of Science
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This is a student project, submitted for academic credit. WPI typically publishes MQP Reports without peer review. For more information about the project program, please visit: https://www.wpi.edu/academics/undergraduate/major-qualifying-project
Abstract

With the Worcester Polytechnic Institute student body increasing, campus space must be expanded. Salisbury Estates presents an opportunity for redevelopment. Through interviews and research, multiple building layouts were considered, and preliminary designs for a residential and an academic building were finalized. Floor plans, life safety, and cost analysis were conducted. A complete framing plan with structural analysis of beams, columns, and footings was created for the academic building. Deliverables include a report, structural calculations, AutoCAD drawings, and a cost estimate.
Authorship

Due to the nature of the team’s project, the paper was continuously being written as the design work was completed. Every member contributed to the overall writing. The primary authors for each of the major detailed sections was the person who led the primary design work on that topic. These sections were broken up as follows:

Zachary Abbott:

Floor Layout Design, Beam and Girder Design, Cost estimating, Preliminary author in tandem with others design

Mark DellaCroce:

Floor Layout Design in AutoCAD, Building egress design and analysis, Determined building occupant loads, Fire Sprinkler layouts in AutoCAD

Dylan Felty:

Interview Coordinator, Spreadsheet manufacturer, Column design, RISA 2D design, Lateral Bracing design, Cost estimating

Tyler Kornacki:

Site layout design, Floor Layout design, Beam design, Stairway and Elevator design, Lateral Bracing Design, Foundation Design
Capstone Design Statement

To satisfy the requirements of the Accreditation Board for Engineering and Technology (ABET) for Capstone Design Projects, the team considered realistic constraints. This section details how the project work addressed these constraints.

Constructability

It is important constructability throughout the design of a project. The design process can be followed correctly, and a structure may work on paper, but if the structural components cannot be integrated successfully then the work done is inadequate. The team continuously consulted outside sources, such as the Massachusetts State Building Code (9th edition) to address factors such as zoning, regulations, design aspects, and structural analysis. Steel sections were taken from the Manual of Steel Construction, published by The American Institute of Steel Construction.

Social

The social impact of a project must be considered to ensure it is actually feasible and successful for the region in which it is being completed. The new facilities will alter the landscape of WPI. Adding the academic facility will provide more classroom and laboratory space for the growing undergraduate class to collaborate and work as well as offices for the additional administration needed to facilitate this learning. The residential facility will also contribute to the support of the growing undergraduate student body by providing much needed dormitory space. The addition of these functional facilities will promote a more integrated campus that expands beyond just the hill on which most activities take place.

Economic

To evaluate the economic feasibility of this project, material and labor cost estimates were prepared. Given that this project will be funded by a private institution, every aspect from design through construction was evaluated.

Healthy and Safety

Health and safety should be considered for all phases of a project’s life. In this case, the construction and occupancy were considered. The team ensured the safety of the construction process, the structure, and its occupants by designing in accordance with the Massachusetts State Building Code 9th edition and American Society of Civil Engineers (ASCE). The structures will be accessible and safe for all its occupants. The location of the facilities subjects them various environmental factors such as earthquake, snow, and wind loads which were accounted for in design.

Ethics

The American Society of Civil Engineers (ASCE) says that “Ethics is integral to all decisions, designs, and services performed by civil engineers.” There are ethical specifications
that must be addressed for every project: designing the project in the best interest of the client, being truthful in the cost and timeline for the project, and not using substandard materials or techniques to save money. By adhering to these procedures, in addition to ASCE’s assertion that “engineers uphold and advance the integrity, honor, and dignity of the engineering profession by using their knowledge and skill for the enhancement of human welfare and the environment, being honest and impartial and serving with fidelity the public, their employers and clients, striving to increase the competence and prestige of the engineering profession, and supporting the professional and technical societies of their disciplines” (ASCE, 2017), this project was completed ethically and appropriately.
**Professional Licensure Statement**

In the state of Massachusetts, as well as the rest of the United States, any construction designs must have the stamp of approval from a licensed Professional Engineer (PE). Because of the necessity to have these stamps of approval obtaining a PE certification is a major step for any engineer looking to further their career. Not only will this licensure open up the possibility for promotions within one’s company but will also invite the opportunity for pay raises.

Due to the importance of a PE licensure, and the repercussions that may come with being held responsible for one’s designs, they are both difficult and time consuming to obtain. There are three major steps in becoming a PE: receiving your college degree, becoming an EIT, and passing the Principles and Practice of Engineering exam. While this might not sound like much at first glance, they all have their own stipulations attached. The first step of obtaining your college degree must come from a 4-year ABET accredited institution. When nearing graduation from this program, or after graduation, one must register for and pass a Fundamentals of Engineering Exam to become an EIT. This is a 110 question computerized test that lasts a total of 5 hours and 20 minutes. These 110 questions are broken down into 18 sections ranging from basic overarching topics like mathematics, statistics, and ethics, to more discipline specific topics of materials, fluids mechanics, and structural design. Upon earning an EIT licensure one must then complete 4 years of professional practice under the supervision of a PE (3 years if having completed a Master’s degree). Once this time has been completed then one can register to take a Principles and Practice of Engineering exam to become a PE. While the FE exam is a nationwide standard that can be taken and accepted throughout the US, a PE is only permitted to work in the state the pass their exam in, and others that accept that states test.

The structures designed in this project would need multiple PE stamps to be erected. While the design drawings shown in the Appendix would need to be stamped by a Civil/Structural PE, all of the nonstructural components would need stamps of approval from a PE in that field (i.e. Architectural, Electrical, Environmental, Fire Protection, and Mechanical).
# Table of Contents

Abstract ......................................................................................................................... i

Capstone Design Statement .......................................................................................... iii

Professional Licensure Statement ................................................................................. v

List of Figures ................................................................................................................ ix

List of Tables .................................................................................................................. x

Executive Summary ...................................................................................................... xi

1. Introduction .............................................................................................................. 1

2.0 Background ............................................................................................................. 2

2.2 Community Impact ............................................................................................... 3

2.2.1 Impact on the WPI Community ................................................................ 3

2.2.2 Impact on the Greater Worcester Community ........................................ 4

2.3 Design Parameters ............................................................................................... 4

2.3.1 Zoning Requirements .................................................................................. 4

2.3.2 Occupancy Classification for Residential Building and Academic Building .... 5

2.3.2 Fire Requirements with Respect to Site Plan ............................................. 8

2.3.3 Fire Requirements with Respect to Building Design ................................. 9

2.5 Sustainability ....................................................................................................... 10

2.6 Ethics .................................................................................................................... 10

3.0 Methodology ......................................................................................................... 12

3.1 Preliminary Information ...................................................................................... 12

3.2 Defining Site Plan ............................................................................................... 13

3.3 Evaluation of Alternatives ................................................................................ 13

4.0 Architectural Floor Plans ...................................................................................... 15

5.0 Fire Safety ............................................................................................................. 17

5.1 Egress Analysis .................................................................................................... 17

5.1.1 Occupant Load ............................................................................................ 17

5.1.2 Egress Width Requirements ..................................................................... 18

5.2 Fire Protection Systems ..................................................................................... 19

6.0 Structural Design for Gravity Loads .................................................................. 20

7.0 Design of Academic Facility ............................................................................... 26

7.1 Roof/Floor Beam and Girder Design .................................................................. 26

7.2 Columns ............................................................................................................... 30
Appendix I: Earthquake Load Excel Document Design Aid .................................................. 119
Appendix J: Wind Load Excel Document Design Aid ......................................................... 121
Appendix K: Building Story Drift Excel Document Design Aid ....................................... 122
Appendix L: RISA 2D Building Models for Lateral Reinforcement Design ..................... 123
Appendix M: Baseplate and Footing Design Hand Calculations ...................................... 125
Appendix N: Cost Estimate of Academic Structural Frame ............................................. 131
Appendix O: Cost Estimate of Academic Non-Structural Elements ................................ 132
List of Figures

Figure 1: Aerial view of Salisbury Estates site – (Google, n.d.) .................................................. 3
Figure 2: Final Site Plan .................................................................................................................. 14
Figure 3: Sample of Residential Building Plans ......................................................................... 15
Figure 4: Sample of Sprinkler Design Drawings ........................................................................ 19
Figure 5: Typical Span-based Depths for Steel Roof Decking, The Architect’s Studio Companion .................................................................................................................. 21
Figure 6: Selection of Metal Roof Deck, Vulcraft Steel Deck Catalog ........................................ 21
Figure 7: Selection of Composite Floor Deck, Vulcraft Steel Deck Catalog ............................... 22
Figure 8: Example Framing Plan Around Staircase and Elevators ............................................. 23
Figure 9: Main Staircase Framing Plan ......................................................................................... 24
Figure 10: Safety Code for Elevators and Escalators, ASME A17.1-2007 pg 242 ....................... 25
Figure 11: Typical Span-based Depth for Steel Trusses, The Architect’s Studio Companion ... 26
Figure 12: Span Ranges for Structural Systems, The Architect’s Studio Companion .......... 27
Figure 13: Level Two Framing of WPI Gateway Park, Gateway Building Plans ..................... 28
Figure 14: Special Framing Around Egress Staircase ................................................................. 29
Figure 15: Special Framing Around Main Staircase .................................................................. 29
Figure 16: Column Bays Investigated for Design ...................................................................... 30
List of Tables

Table 1: Design Parameters/Regulatory Considerations ......................................................... 5
Table 2: Minimum Building Code Requirements for Building Design ..................................... 6
Table 3: Building Code Requirements for Fire Safety ............................................................... 8
Table 4: Building Code Requirements for Life Safety Considerations .................................... 10
Table 5: Interview Takeaways .................................................................................................. 13
Table 6: Occupant Loads ......................................................................................................... 17
Table 7: Calculated Egress Width ............................................................................................. 18
Table 8: Governing Live Load Values ...................................................................................... 20
Table 9: Lateral Reinforcement Member Sizes ........................................................................ 32
Table 10: Baseplate and Footing Dimensions and Reinforcement ........................................... 33
Table 11: RSMeans Upscaled Member Sizes ............................................................................ 35
Table 12: Fire Protection Costs ............................................................................................... 36
Table 13: Total Academic Building Cost Estimate ................................................................. 36
Table 14: Total Residential Building Cost Estimate ............................................................... 38
Executive Summary

This Major Qualifying Project (MQP) centered around the acknowledgement that as Worcester Polytechnic Institute (WPI) student body continues to grow, the need for more residential, academic, and collaborative spaces is present. This report offers a solution to the current housing and classroom space shortages that will only become more of a concern as incoming class sizes continue to increase. The comprehensive redesign of the Salisbury Estates property has involved the demolition of all existing buildings, roadways, paths, and landscape to allow for a new complex of large residential and academic buildings as well as an additional dining facility. This report includes AutoCAD drawings of a proposed academic building and a residential building as well as a site plan for the entire Salisbury Estates property, and a cost estimate for the undertaking of this project. These proposed buildings offer a significant addition of classrooms, common areas, dormitories, and parking spaces needed to accommodate the growing undergraduate population. Areas of depth have included structural design and analysis as well as life safety and fire protection, all of which have been addressed in accordance with applicable codes, standards, and Worcester City Ordinances.

Through multiple conducted interviews, as many interests and needs were included based on WPI employee feedback. The following functionalities have been incorporated in the design of the residential and academic buildings. For the residential building design, meeting spaces, large common areas and tech suites are provided. These functional spaces will promote collaboration and increase the appeal of the all new redesigned complex. The design incorporates housing for several hundred students within a two resident per unit style dormitory facility reaching a total of three stories. The first floor includes a dining area and a connecting lounge area available to members of the WPI community. The residential building forms a U-shape and has a total make up of 104,100 square feet.

The academic building design includes a combination of lecture halls, classrooms, offices, and tech suites in order to appeal to the several needs of the institution and create a greater draw to the development to complete work and collaborate. This building was designed to be three stories and has a total make up of 75,000 square feet.

The parking arrangement has been kept similar to the existing Salisbury Estates layout including street and flat lot style parking. Salisbury Estates occupies a substantial amount of land, some of which has been left for development outside of the building construction. A balance has been determined regarding what is to be developed into parking and what is to be set aside to form an open, green space that connects the facilities, similar to the current Quadrangle on campus. In addition to providing students a safe place to go outside and enjoy leisurely activities, this open space will also provide opportunity for further development in the future if necessary.
The architectural layouts were completed for both buildings while a complete structural design and analysis was completed for the academic building with similar properties and methods used for cost estimates of the residential building as time did not allow for a complete structural analysis of both buildings. Upon completing the floor layout for the academic building, the columns were placed as accurately and most desirable as possible while fitting the floor layout. Using beam and girder sizing design aids, the allowable distance separating the columns was determined and member lengths were chosen. Once the bay sizes were determined, ASCE 7 was used to determine design loads for sizing of members. A top-down approach to size members was taken, starting with the roof bays, followed by the third floor, then second floor, and ending with the first floor. With beams and girders sized and self-weights determined, the columns supporting these bays were then designed. The framing plan was finalized with the design of lateral bracing in the corner bays. When the framing designs were completed, foundation design began. Due to the lack of a basement in this building, the foundation design involved simple baseplates and footings for each column.

Simultaneously to the structural design, a life safety analysis was conducted while the architectural drawings were being finalized for both buildings. The egress analysis ensured there was enough space present in the architectural drawings for building occupants to safely egress as well as proper door swing with respect to occupant load in each room of the building. Following this, fire protection plans were made in the form of AutoCAD plans for the installation of full automatic sprinkler systems for both buildings.

An accurate cost estimate for first the academic building was assessed using cost data from RSMeans publications. Following the completion of the structural design, cost data from RSMeans publications was consulted for costs per linear foot of all structural members. In addition to these linear foot values, all nonstructural elements were estimated with cost per square foot values. Using these same methods the cost estimate for the residential building was also developed. Fire protection costs were estimated on a square foot basis for both buildings with respect to sprinkler and fire alarm costs.

Future recommendations have been included as to how this project can be continued with future MQP teams. This completed project includes enough information and drawings to demonstrate that a new academic and residential building is a useful way to repurpose the existing Salisbury Estates property while filling a great need to the WPI campus with the addition of dormitory, classroom, lecture hall, office, tech suite, dining, and collaborative spaces for students to live and grow as individuals.
1. Introduction

The following has been derived from what Worcester Polytechnic Institute (WPI) lacks most in terms of infrastructure on campus. This report offers a solution to the current housing and classroom space shortages that will only become more of a concern as incoming class sizes continue to increase. The comprehensive redesign of the Salisbury Estates property has involved the demolition of all existing buildings, roadways, paths, and landscaping to allow for a new complex of large residential and academic buildings as well as an additional dining facility. This report includes a digital model of the proposed buildings, a site plan for the entire site, and a cost estimate. This will also demonstrate the significant addition of classrooms, common areas, dormitories, and parking spaces provided to accommodate the growing undergraduate population. Areas of depth have included structural design and analysis as well as life safety and fire protection, all of which will have been addressed in accordance with applicable codes and standards.

The residential building design includes meeting spaces and large common areas as well as tech suites. These functional spaces will promote collaboration and increase the appeal of the complex. The design incorporates housing for several hundred students within a two-resident-per-unit style dormitory facility. The first floor also includes a dining area and a connecting lounge area available to members of the WPI community. The separate academic building design includes a mixture of lecture halls, classrooms, offices, and tech suites in order to appeal to the several needs of the institution and create a greater draw to the development.

More parking is necessary to accommodate the new users of the facilities including faculty, staff, and students. It has been deemed undesirable to erect an entire parking garage for this purpose. Therefore, the parking has been kept similar to its current style with street and lot parking. Salisbury Estates occupies approximately 9 acres, some of which has been left for development outside of the building construction. A balance has been determined regarding what is to be developed into parking and what is to be set aside to form an open, green space that connects the facilities, similar to the current Quadrangle on campus. In addition to providing students a safe place to go outside and enjoy leisurely activities, this open space will also provide opportunity for further development in the future if necessary.
2.0 Background

This project aimed to provide more residential space to account for the growing WPI student body, as well as an academic facility to further support this growing community and promote more integration of the Salisbury Estates area with the rest of campus. To effectively deliver this project, certain background knowledge was required. Several factors were considered for the design of these new structures. The site was first assessed to determine property lines and land conditions that would restrict the scope of work. Design ideas were then developed with input from WPI faculty and staff to ensure that the wants and needs of the University were considered. Building code research was conducted to ensure that the proposed buildings have been designed in accordance with applicable codes and standards. The project’s impact on the community was also considered to ensure that the new development would fit well into the area and not cause issues with surrounding properties. Designs are only accepted if they are economically feasible; therefore, the cost of the proposed development has been assessed. Decisions were also made ethically, and items were addressed in a correct, professional manner throughout the project.

2.1 Site Information

The proposed buildings have been situated within the lot of WPI’s Salisbury Estates. This area is located along Massachusetts Route 122A (Park Avenue), between Salisbury Pond and the Worcester Center for Crafts as seen in Figure 1. For students to access this complex by foot, they must either walk along an indirect sidewalk or an unpaved and unlit path along Salisbury Pond. Alternatively, access by car is only possible through a single entrance and exit on Park Avenue (Rumford Avenue). The plot of land is quite expansive and has primarily level topography. According to flood maps from the FEMA Flood Map Service Center, this land is in an area of minimal flood risk despite its close proximity to Salisbury Pond; therefore, flooding is of no issue.

In addition to the plot of land Salisbury Estates is currently on, WPI owns some additional neighboring property. The large building located between the Worcester Center for Crafts and Avis Car Rental belongs to WPI and is currently being used to store equipment for WPI Facilities. Aside from this building, all of the land between the Worcester Center for Crafts and Grove Street is under WPI control. This includes a small parking area to the north that has been expanded southward to provide additional parking and a through way for an extra access point to the Salisbury Estates property.
2.2 Community Impact

The redesign of the Salisbury Estates property will affect the surrounding Worcester community. From the demolition of the existing property to the active construction of the buildings, the proposed development will impact not only the WPI community, but also the community of Worcester.

2.2.1 Impact on the WPI Community

With student populations at WPI growing every year, on-campus residential space has reached its capacity; all available rooms have been filled and some students are even on housing waitlists. WPI administrators and staff from across the institution agree that additional housing for undergraduates is the top priority moving forward. Without access to on-campus housing, students are forced to move to off-campus alternatives which can vary in quality and safety. In extreme cases, some students may even resort to lengthy or inconvenient commutes from neighboring communities if they are unable to secure housing on campus.

Although residential space has been identified as the top priority for future development according to interviews, the need for additional academic space is also present. As the size of the student body grows, so does the amount of classes offered. With only so many hours in a day,
WPI has begun to encounter scheduling difficulties due to a lack of available classrooms for professors to instruct courses. The growing student body will only intensify this issue in coming years if additional classroom space is not added to campus.

2.2.2 Impact on the Greater Worcester Community

The new construction on Salisbury Estates has significant potential to impact the greater Worcester community. In order to begin the construction for the project, all existing structures will be demolished and landscape cleared. From this initial step, this project will impact the greater Worcester area. With demolition and new construction comes loud noise, displacement of existing residents, and potential impediment of public utilities for surrounding buildings, all of which will affect residents of the community for the duration of the project. Additionally, once built, the new structures will themselves become part of the community. The structures have been designed to fit into the theme of the surrounding area and not stick out as loud additions to the pre-existing neighborhood.

2.3 Design Parameters

The following sections discuss the technical aspects of the team’s final building designs and what has been incorporated with respect to fire code requirements and occupancy classifications. The limiting factor for the design of the building lies in the codes, standards, and ordinances put forth by the City of Worcester and the Commonwealth of Massachusetts. The building has been designed according to the *2015 Edition of the International Building Code* (IBC) which Massachusetts currently adopts, the *9th edition of the Massachusetts State Building Code* (MSBC), which Massachusetts currently sets in place to amend certain sections of the 2015 IBC, all National Fire Protection Association (NFPA) codes and their respective editions adopted by Massachusetts, and all zoning and ordinances for Worcester.

2.3.1 Zoning Requirements

Across Massachusetts, cities and towns are divided into different zoning districts that serve to regulate the use of specific plots of land and govern building characteristics across different neighborhoods and usage areas. Although the entirety of WPI’s main campus is zoned as Institutional (IN-S), it was discovered that the Salisbury Estates property has been zoned as Limited Residential (RL-7) which presented several design restrictions that do not apply to WPI’s campus on the hill. Of significant relevance to this project are the restrictions on permitted land use, permitted dimensions, and off-street accessory parking requirements. These regulations can all be found within Tables 4.1, 4.2, and 4.4 respectively of the City of Worcester Zoning Ordinance. Table 1 shows the major considerations associated with each regulation that were taken into account during the preliminary discussion of design alternatives.
Referring to these regulations impacted the scope of alternative designs discussed. Initially, the design was to incorporate a building height of five stories; however, the Worcester Ordinances identified that the maximum height restriction for the specified zone was limited to three stories. This research also provided a clearer picture of the amount of parking required based on the number and types of occupants.

Further investigation revealed that there exists a 100-foot buffer zone from the waterline of Institute Pond which meant that all new construction must be set back 100 feet from the pond. If construction was desired beyond the 100-foot buffer zone, then a variance must be requested to receive permission.

### 2.3.2 Occupancy Classification for Residential Building and Academic Building

The team decided to propose a three-story residential hall with bedrooms, common spaces, and tech suites. The academic building design has incorporated classrooms, laboratories, lecture halls, offices, and tech suites. A dining facility was also implemented on the 1st floor connecting the two halls in the center. Table 2 shows the code requirements and restrictions that were taken into account for the proposed building designs. These occupancy classifications were important to determine the appropriate occupant load of the various spaces in the buildings once final layouts and areas were discussed and agreed upon.
<table>
<thead>
<tr>
<th>Occupancy in Question</th>
<th>Code Requirement</th>
<th>Code Reference</th>
</tr>
</thead>
<tbody>
<tr>
<td>Residential Hall Occupancy Classification</td>
<td>Residential Group R-2: sleeping spaces of more than two dwelling units where occupants are primarily in nature, which includes dormitories.</td>
<td>MSBC Section 310.4.</td>
</tr>
<tr>
<td>Occupancy Separation</td>
<td>Group R-2 occupancies shall be separated from other accessory occupancies.</td>
<td>MSBC Section 508.2.4</td>
</tr>
<tr>
<td>Need for Automatic Sprinkler System</td>
<td>Group R occupancies shall be equipped throughout with an automatic sprinkler system.</td>
<td>MSBC Section [F] 420.5</td>
</tr>
<tr>
<td>Entry common space occupancy classification</td>
<td>Assembly Group A, a portion of a building used for gathering of persons for purposes including recreation shall be classified as Assembly.</td>
<td>MSBC Section 303.1</td>
</tr>
<tr>
<td>Dining hall occupancy classification</td>
<td>Assembly Group A-2, use of cafeterias and similar dining facilities.</td>
<td>MSBC Section 303.3</td>
</tr>
<tr>
<td>Commercial kitchen occupancy classification</td>
<td>Assembly Group A-2, associated commercial kitchens attached to a dining facility.</td>
<td>MSBC Section 303.3</td>
</tr>
<tr>
<td>Offices</td>
<td>Business Group B Occupancy</td>
<td>MSBC Section 302</td>
</tr>
<tr>
<td>Tech Suites</td>
<td>Group B Occupancy because Tech Suites are intended to hold less than 50 people</td>
<td>MSBC Section 302</td>
</tr>
<tr>
<td>-------------</td>
<td>--------------------------------------------------------------------------------</td>
<td>------------------</td>
</tr>
<tr>
<td>Classrooms and lecture halls</td>
<td>Assembly Group A</td>
<td>MSBC Section 303.1</td>
</tr>
</tbody>
</table>

The residential spaces have remained separate from the main entryway with the attached dining facility; furthermore, for security reasons, double door vestibules with key card access from the ingress side of the building entryway to the residential areas have been incorporated to prevent unwanted persons from entering the residential halls. The occupancy classifications were used to calculate the occupant loads which can be present at any given time in the residential space, entryway common space, dining space, and commercial kitchen space. The determined occupant loads are set forth by the code to ensure the buildings can support a safe evacuation in the event of a fire. If the total number of building occupants exceed the occupant load of the building, staircases and doorways will become overcrowded inhibiting a safe egress for all building occupants.

2.3.2.1 Building Construction Type

Type I and Type II construction were considered for the residential building, ideally including a full steel structural frame with brick facade. Type II construction was decided upon due to the primary focus of the project being on the design of the structural frame, that being of steel. The construction type and occupancy classifications were then used to calculate the maximum allowable building footprint per the MSBC. Type I construction is defined by the use of steel, and Type II construction is defined by the use of brick or masonry with steel structural member elements. Types III, IV, and V construction include the use of combustible materials and are therefore undesirable for use in a residential dormitory building.

2.3.2.2 Building Size Limitations

Table 3 shows the building size limits with respect to construction type. Since the proposed buildings are to be comprised of mainly steel structural elements, the steel is required to have a certain level of fire resistance in an hour rating system. Table 3 shows the requirements per the MSBC.
Table 3: Building Code Requirements for Fire Safety

<table>
<thead>
<tr>
<th>Occupancy in Question</th>
<th>Code Requirement</th>
<th>Code Reference</th>
</tr>
</thead>
<tbody>
<tr>
<td>Building Height allowance</td>
<td>85 feet for residential occupancies of Type II construction when equipped with an automatic sprinkler system</td>
<td>MSBC Table 504.3</td>
</tr>
<tr>
<td>Stories permissible</td>
<td>5 stories under Type II construction when equipped with an automatic sprinkler system</td>
<td>MSBC Table 504.4</td>
</tr>
<tr>
<td>Building area allowance</td>
<td>Unlimited under residential occupancies of Type II construction, provided the building is equipped with an automatic sprinkler system</td>
<td>MSBC Table 506.2</td>
</tr>
<tr>
<td>Type II structural elements</td>
<td>1-hour fire resistance rating for all structural members of the building frame</td>
<td>MSBC Table 601</td>
</tr>
<tr>
<td>Bearing walls</td>
<td>1-hour fire resistance rating</td>
<td>MSBC Table 601</td>
</tr>
<tr>
<td>Roof Structure</td>
<td>1-hour fire resistance</td>
<td>MSBC Table 601</td>
</tr>
</tbody>
</table>

2.3.2 Fire Requirements with Respect to Site Plan

The following sections contain the limits to the site plan design with respect to fire requirements including the fire access road, the location of the entrances to the buildings, and the fire department connections to the buildings. These requirements assisted in shaping the landscape of the site plan and identifying locations of parking spaces.

2.3.2.1 Fire Department Connections

The design of both the residential and academic buildings had to account for the location of fire department connections. MSBC Section [F] 912: Fire Department Connections states that with respect to hydrants, driveways, buildings and landscaping, these fire department
connections shall be located so that fire apparatus and hoses connected to supply the system will
not obstruct access to the buildings for other fire apparatus. The fire department connections
shall be located on the street side of buildings, fully visible, and recognizable from the street or
nearest point of fire department vehicle access. The section continues to acknowledge that the
fire department connections shall be maintained at all times and shall never be obstructed by
fences, bushes, trees, walls or any other fixed or moveable object.

2.3.2.2 Fire Access Road

According to MSBC Section 503.1.1, approved fire apparatus access roads shall be
provided for every facility, building, or portion of a building hereafter constructed or moved into
or within the jurisdiction of the project. The section further explains that the fire access road
shall extend to within 150 feet of all portions of the facilities and all portions of the exterior walls
of the first stories as measured by an approved route around the exterior of the buildings. It is
also noted that the fire access road shall have an unobstructed width of no less than 20 feet, and
an unobstructed vertical clearance of no less than 13 feet-6 inches.

2.3.3 Fire Requirements with Respect to Building Design

Table 4 addresses design codes and specifications regarding occupancy loading, egress
plans, sprinkler systems, as well as the materials and furnishings that the team had to abide by in
designing specific components of the buildings.
<table>
<thead>
<tr>
<th>Category of Consideration</th>
<th>Code Requirement</th>
<th>Code Reference</th>
</tr>
</thead>
<tbody>
<tr>
<td>Occupancy Load Calculations</td>
<td>Occupant load placards shall be provided for each space of the buildings.</td>
<td>MSBC 1607.1</td>
</tr>
<tr>
<td>Egress Plan</td>
<td>Evacuation plans shall be provided for each level of the buildings demonstrating primary and secondary means of egress</td>
<td>NFPA 101 2013 Edition</td>
</tr>
<tr>
<td>Sprinkler Design</td>
<td>Building shall be sprinklered throughout</td>
<td>NFPA 13</td>
</tr>
<tr>
<td>Building Entrance Access</td>
<td>Building entrance access shall be within certain distance of the fire access road, through which the main fire alarm panel shall be accessible</td>
<td>MSBC 504.1</td>
</tr>
</tbody>
</table>

### 2.5 Sustainability

Projects aiming to improve infrastructure must be effective not only today but also for their impact in the future. In order to accomplish this, sustainability must be addressed in design. It was important to assess the conditions of the site itself and design to minimize the project’s impact on the plot of land due to environmental concerns such as flooding and erosion. The proposed redesign of Salisbury Estates is environmentally friendly and sustainable to accommodate for future generations as the WPI community continues to grow. The sustainability of the building materials and construction processes used were addressed along with the design itself.

### 2.6 Ethics

Throughout this project several ethical considerations were kept in mind. The American Society of Civil Engineers (ASCE) says that “Ethics is integral to all decisions, designs, and services performed by civil engineers.” There are ethical specifications that must be addressed for every project: designing the project in the best interest of the client, being truthful in the cost and timeline for the project, and not using substandard materials or techniques to save money. Worcester residents of Salisbury Estates who would have to be relocated upon construction were also considered. These residents would be alerted at the beginning of the planning process to provide them with ample time to find a new residence; assistance for these residents will also be
provided by WPI. This affected the decision of the construction method. Design-bid-build was used instead of design-build to give the residents more time to relocate under less pressure. By adhering to these procedures, in addition to ASCE’s assertion that “engineers uphold and advance the integrity, honor, and dignity of the engineering profession by using their knowledge and skill for the enhancement of human welfare and the environment, being honest and impartial and serving with fidelity the public, their employers and clients, striving to increase the competence and prestige of the engineering profession, and supporting the professional and technical societies of their disciplines” (ASCE, 2017), this project was completed ethically and appropriately.
3.0 Methodology

3.1 Preliminary Information

The first primary objective of the project was to determine what structural design was to be undertaken. A discussion with project advisor Professor Leonard Albano addressed design projects that could be continued upon and new projects that WPI has actually been considering. This gave rise to the idea of redesigning the apartments of Salisbury Estates. In order to collect further information regarding this on-campus complex, meetings were held with Residential Services and Facilities staff.

Meetings were first held with Matthew Foster and Amy Beth Laythe from Residential Services. These two individuals were sought out because of their roles in Residential Services and the knowledge their roles provided regarding life on campus. They both spoke heavily of the current conditions and issues regarding Salisbury Estates, which would later be affirmed by visiting a student resident at the complex. Additionally, these discussions exposed more undergraduate housing as being WPI’s current greatest need. The WPI Residential staff shared potential means of developing and improving the complex in order to address these concerns. The details of these potential avenues of pursuit were discussed in order to better define what the school would want with this Major Qualifying Project and what could actually be produced within the span of one academic year. This allowed for the identification of a project topic that would be both feasible and effective.

Another meeting held with Bill Spratt of Facilities affirmed the information gathered from Residential Services. It was agreed upon that residential space is currently the greatest need at WPI. The poor state of Salisbury Estates was discussed from the facilities perspective as well. It was shared that the complex was nearing the point of being dysfunctional due to its deferred maintenance. This has been a recurring issue since less than 50 percent of the occupants are WPI residents and the school technically does not need to provide regular maintenance as they do for other facilities on campus that are strictly occupied by WPI. Mr. Spratt also shared the idea of implementing the addition of more dining space into the redesign of the complex. Table 5 lists the key points taken from each interview.

As stated above, in order to further understand the current conditions of Salisbury Estates, the team also visited the site and toured the apartment of a WPI resident. This allowed for the collection of first-hand observations of the interior spaces as well as the overall layout of the complex and potential for development. The lack of maintenance mentioned through the previous interviews was certainly noticed and the need for renovation and improvement was shared by the WPI resident visited by the team.
Table 5: Interview Takeaways

<table>
<thead>
<tr>
<th>Interviewee</th>
<th>WPI Department</th>
<th>Key Points</th>
</tr>
</thead>
</table>
| Matthew Foster       | Residential Services | ● Focus on undergrad  
                        |                                                               | ● Residential/academic mixed-use  
                        |                                                               | ● Open meeting space       |
| Amy Beth Laythe      | Residential Services | ● More dining  
                        |                                                               | ● Separate residential & academic  
                        |                                                               | ● Integrated with rest of campus/community |
| William Spratt       | Facilities       | ● Need for improvement affirmed  
                        |                                                               | ● Not top priority since majority of tenants non-WPI |

3.2 Defining Site Plan

For the residential facilities there will be one U-shape building with a wedge/dining area in the middle, similar to Morgan and Daniels Hall. The wings of the U-shape will be mirror images of each other, each having an L shape and being connected by the wedge on the first floor center. Opposite the resulting U shape will stand a rectangular academic facility, somewhat forming an open space similar to the Quadrangle on campus today.

After the completion of the site layout design it was decided by the team to dedicate time and resources to the academic building initially. The reason for this was due to the complexity in the layout of the residential building. It was realized early on that connecting the wings of the U-Shape to the main body was a problem the team had not encountered before and therefore design of this structure would prove far more time consuming. Due to the limited time to work on the design of these facilities, the team’s time would be better used on the academic building instead of researching this problem. At the conclusion of the academic building design, the gathered information was extrapolated to determine an estimated cost for the residential building.

3.3 Evaluation of Alternatives

Prior to deciding upon a final site layout for the Salisbury Estates property, many alternative options were investigated as shown in Appendices B-E. Appendix B shows a sketch with three separate facilities, all of which were deemed too large after consulting maps of the site and appropriate distance scales. The building drawn in Appendix C was decided against due to
the awkward connection angles which would cause unnecessary complications in design later on. The northerly facing U-shape building was decided upon as shown in Appendix D. This demonstrates the desire to use the academic and residential buildings to make an enclosed open area similar to the Quadrangle. Further investigation into the boundary requirements led to the change of the academic building from a U-shape to a normal rectangle, which also opens up additional space to further develop in the future if necessary.

Through the evaluation of design options and consideration of limiting design specifications, the scope of the project regarding the structures to be designed and layout of the complex was decided upon. It was determined that the setback on the property line to the North was closer than thought, so the dining area was moved into the wedge. In the end the team decided to design one residential facility joined by a new wedge/dining space on the first floor along with a separate academic facility across an open space similar to the Quadrangle currently on campus. The layout of these buildings can be seen in Figure 2 in which blue and yellow represent the residential halls and academic facility, respectively.

Figure 2: Final Site Plan
4.0 Architectural Floor Plans

When developing the floor plans for the academic and residential buildings a similar design method was followed. Each began with an initial idea of what was desired for the building, then as they were modeled around those ideas, the layouts were adapted to accommodate what was required based on fire safety and commercial building needs. The buildings were drawn in AutoCAD using different line colors for ease of viewability. A section of the residential building plans can be seen in Figure 3.

Aside from these more detail-oriented changes there were some large adaptations that were necessary to make the limited spatial layouts and floor plans in both buildings. The residential building was the first to be looked through and finalized due to its unique style. One of the areas this building needs to house is some form of dining. This was originally designed to be located in an extension off the back end of the building. However, further investigation of the setback requirements of the lot showed that this extension would be outside of the permissible construction zone. As a result the dining portion of the building was moved into the wedge of the first floor connecting the two wings. The wedge was the location of the second major change in the residential building. When looking into dividing out the wedge into its different sections it was realized that the area allocated to this sole space that could be better utilized for more dorm rooms. This realization led to the shrinking of the wedge lengthwise on both sides, allowing for five more dorm rooms, along with a trash room, in each first floor wing. The final major change that was made in the residential building was a result of the fire safety analysis. When the floor
layouts were originally completed an occupancy rating was calculated. This showed that a second egress stair had to be added at the end of each wing next to the common rooms. These stairs were placed in the spaces previously assigned to dorm rooms, therefore resulting in the loss of two rooms.

After the finalization of the residential building floor layout the team moved to finalizing the floor plans for the academic building. The large changes to the academic building include revising the architectural drawings to eliminate dead-end corridors from the building, and adding egress doors that lead directly to the exterior of the building from the four large lecture hall spaces. The overall design of the building was completely changed to minimize any wasted spaces to incorporate as many areas for students to study and collaborate with one another as possible. A central main staircase was added to the building which is accessible from the entrances at either end of the building for an open concept feel. Four elevators are proposed throughout the building for ease and convenience of mobility. Both final architectural drawings can be found in Appendix E.
5.0 Fire Safety

The fire safety section involves the quantification of life safety elements including how long it will take building occupants to escape the building in the event of a fire, how many people can be in each building at any given time, and how wide the escape paths have to be for building occupants. Finally, fire protection systems have been designed to increase life safety and to proactively suppress fires.

5.1 Egress Analysis

A full egress analysis was conducted upon completion of the architectural drawings of both buildings for the purpose of ensuring that all occupants could safely escape in a fire event.

5.1.1 Occupant Load

To conduct an egress analysis, first the occupants loads for each room and each floor had to be developed to determine the number of enclosed fire rated egress stairwells that were needed to be incorporated into both buildings. Table 6 shows the occupant load of each floor for both the academic and residential buildings.

<table>
<thead>
<tr>
<th>Building and Floor Level</th>
<th>Maximum Occupant Load</th>
</tr>
</thead>
<tbody>
<tr>
<td>Academic Building First Floor</td>
<td>926 Occupants</td>
</tr>
<tr>
<td>Academic Building Second Floor</td>
<td>762 Occupants</td>
</tr>
<tr>
<td>Academic Building Third Floor</td>
<td>451 Occupants</td>
</tr>
<tr>
<td>Residential Building First Floor</td>
<td>678 Occupants</td>
</tr>
<tr>
<td>Residential Building Second Floor</td>
<td>270 Occupants</td>
</tr>
<tr>
<td>Residential Building Third Floor</td>
<td>270 Occupants</td>
</tr>
</tbody>
</table>
5.1.2 Egress Width Requirements

The MSBC requires that each occupant within a building is provided with 0.3 inches of clear width for egress staircases. This factor is used to determine how many staircases are needed and how wide each of these staircases must be. Table 7 shows the total width of egress that must be provided per floor of each building using the multiplier of 0.3 inches per occupant. Further all other components of egress such as the clear width of doorways must be provided with 0.2 inches of clearance per occupant in accordance with the MSBC. The architectural building design layouts offer more space in width than code requires as seen in Table 7.

<table>
<thead>
<tr>
<th>Building and Floor Level</th>
<th>Egress Width Required</th>
<th>Number of Staircases or Egress exits provided</th>
<th>Egress widths supported by proposed design</th>
</tr>
</thead>
<tbody>
<tr>
<td>Academic Building First Floor</td>
<td>277.8 Inches</td>
<td>4 Egress Stairwells</td>
<td>280 Inches</td>
</tr>
<tr>
<td>Academic Building Second Floor</td>
<td>228.6 Inches</td>
<td>4 Egress Stairwells</td>
<td>280 Inches</td>
</tr>
<tr>
<td>Academic Building Third Floor</td>
<td>135.3 Inches</td>
<td>4 Egress Stairwells</td>
<td>280 Inches</td>
</tr>
<tr>
<td>Residential Building First Floor</td>
<td>203.4 Inches</td>
<td>4 Egress Stairwells</td>
<td>280 Inches</td>
</tr>
<tr>
<td>Residential Building Second Floor</td>
<td>81 Inches</td>
<td>4 Egress Stairwells</td>
<td>280 Inches</td>
</tr>
<tr>
<td>Residential Building Third Floor</td>
<td>81 Inches</td>
<td>4 Egress Stairwells</td>
<td>280 Inches</td>
</tr>
</tbody>
</table>
5.2 Fire Protection Systems

Automatic sprinkler systems were designed in accordance with NFPA 13, the sprinkler installation code, to ensure full sprinkler coverage of both the Academic and Residential buildings. Appendix F shows detailed layouts of the sprinkler system including head locations, riser locations, branch pipe locations, and pipe sizing.

The buildings will be equipped with concealed pendant style sprinkler heads to offer the cleanest finish to each room and to give an optimally modern aesthetic. Sidewall sprinkler heads were used strategically in closets and in the entry vestibules to minimize excessive placement of sprinkler piping. A section of the sprinkler piping layout can be seen in Figure 4. The full sprinkler layout including head placement, piping arrangement and sizing, as well as riser locations can be seen for both buildings in Appendix F.

Figure 4: Sample of Sprinkler Design Drawings
6.0 Structural Design for Gravity Loads

The following section discusses the decision-making processes that went into the selection of governing load values for the structural design of the academic building. Applicable loads were extracted from Chapter 16 Section 1607 (Live Loads) of the 2015 IBC. Based on the types of spaces incorporated into the building, several different live load values were gathered leading to a lot of variation throughout the system. In order to simplify the design process in a conservative manner, it was decided to design members based on the governing live load for each level. These loads can be seen in Table 8.

<table>
<thead>
<tr>
<th>System Component</th>
<th>Governing Live Load</th>
<th>Load Value (psf)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Roof</td>
<td>Snow</td>
<td>55</td>
</tr>
<tr>
<td>Third/Second Floors</td>
<td>Above Corridor</td>
<td>80</td>
</tr>
<tr>
<td>First Floor</td>
<td>First Floor Corridor</td>
<td>100</td>
</tr>
</tbody>
</table>

As prescribed in ASCE 7, a reduction to the uniformly distributed live load was applicable for members supporting substantial tributary area. This applied, however, to only the floor elements. It was noticed that although the applied loads for the floors were greater in magnitude than those for the roof, this live load reduction for the floors actually led to the beam sizes for the roof matching some of those for the floors when looking at the outer 35-foot span sections.

Moving to dead loads, research was conducted to determine the values to be used for the essential slab and deck elements. When calculating the load for the concrete slab the standard 4-inch thickness was used (Concrete Construction, 2018). It was decided to use three-inch metal deck for both the roof and floors. The Vulcraft Steel Deck Catalog was referenced in order to determine which specific type of deck to use for each. 3-inch decking was used for floors and applied to the roof design as well for consistency. This is supported by Figure 5 from The Architect’s Studio Companion which shows that a 3-inch deck suffices for a span range of about 8 to 16 feet, encompassing the beam spacing of the designed roof system (9.33ft-11.50ft). Specific decking was selected based on member spacing and loading parameters. The spacing selected to determine the decking was the greatest beam spacing in the structural system (11’-6”) for both the roof and floors. For the roof, the 3N19 deck was selected since it had an allowable load of 59 psf for one span with spacing of 11’-6” as it sufficed when compared to the governing load of 55 psf as seen in Figure 6. For the floors, the 3VLI22 composite deck was selected based on the superimposed live load of 97 psf for the 3-inch thickness and 11’-6” spacing as shown in Figure 7. Although the greatest floor live load was 100 psf, the concrete used for floor slabs was
115 pcf rather than the 145 pcf the Vulcraft values correspond to which provided sufficient conservativity.

![Figure 5: Typical Span-based Depths for Steel Roof Decking, The Architect's Studio Companion](image)

<table>
<thead>
<tr>
<th>No. of Spans</th>
<th>Deck Type</th>
<th>Max. SDI Const. Span</th>
<th>Allowable Total Load (psf) / Load Causing Deflection of L/240 or 1 inch (psf)</th>
<th>Span (ft-in.) ctc to ctc of supports</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td></td>
<td></td>
<td>10-0</td>
<td>10-6</td>
</tr>
<tr>
<td>N22</td>
<td>11-7</td>
<td>50 / 43</td>
<td>46 / 37</td>
<td>42 / 32</td>
</tr>
<tr>
<td>N20</td>
<td>13-2</td>
<td>66 / 56</td>
<td>60 / 48</td>
<td>56 / 42</td>
</tr>
<tr>
<td>N19</td>
<td>14-7</td>
<td>79 / 69</td>
<td>71 / 59</td>
<td>66 / 51</td>
</tr>
<tr>
<td>N18</td>
<td>15-11</td>
<td>91 / 81</td>
<td>82 / 70</td>
<td>75 / 61</td>
</tr>
<tr>
<td>N15</td>
<td>16-6</td>
<td>118 / 110</td>
<td>107 / 95</td>
<td>97 / 83</td>
</tr>
</tbody>
</table>

![Figure 6: Selection of Metal Roof Deck, Vulcraft Steel Deck Catalog](image)
The exterior enclosure was another element that had to be addressed for the design of the exterior beams and girders. It was determined the exterior facade would be composed of brick to comply with the standard design of campus buildings. The team referred to *The Design of Wood Structures* - Appendix B: Weight of Building Materials (Breyer, 2015) which provided a value of 38 psf. This was factored by an average wall height of 15 ft for each floor of the structure to get an exterior enclosure value of 570 lb/ft.
Stairways and elevators were unique live loads that required extended research and calculations to determine the resultant loading. Once the new framing plans were set so the beams would no longer interfere, it had to be determined what the implications of the beam loading would be. The bays containing the egress stairs contained two components. First was the distributed loads on the new north-south spanning beams. The design of these beams was the same process as the standard vertical beams. Complications arose when it came time to look at the vertical beams the horizontal ones were connected to. These were unique because on one side was the connected beams from the stairway and on the other was the uniform floor loading. While the situation as a whole was unique, when broken down it was two scenarios that had already been analyzed. Figure 8 shows an example illustration of the altered framing of one of these bays.

Figure 8: Example Framing Plan Around Staircase and Elevators
After the completion of the bays for the egress stairwells, design moved to the main stairway. While the main stairway only utilized one E-W beam, making its design more simple than the egress stair, the N-S beams it was connected to posed many questions to be answered. On the other side of these beams were the building elevators along with a small HVAC closet. Elevators are a design topic the team had little experience with. An illustration of this frame can be seen in Figure 9. IBC does not give details on the loading from elevators, but directs readers to the use of ASME A17.1/CSA B44 (ASME, 2016). Consulting with this resource led to the finding of Figure 10 which gives a graph for design concentrated load vs. area. While the original floor plan had the elevators as approximately an 12.5ft by 12 ft area, this graph only gives loads up to a 54 sq. ft. area. Beam spacing in this section of the building was set to be 9.33 ft, so the additional 3.5 ft of elevator was cut back to allow for an entryway to the elevators. Additionally, in order to comply with the 54 sq. ft. max area found in ASME the elevator was cut in half to form two elevators with 9 ft by 6 ft dimensions.

Figure 9: Main Staircase Framing Plan
With the loads of the elevators determined it had to be considered how to apply them. All other loads in the building were used as distributed loading per foot of beam, now the elevators are a concentrated live load. In order to be conservative with the beam design it was decided to place the concentrated loads at the points that would cause the greatest moment and deflection in the beams. Along with these concentrated loads from the elevators and the supporting beam from the main staircase there was a small partially distributed load load from the HVAC closet. Once all of the loads and deflections on this beam had been determined and combined, the final deflection was arrived at. While typical deflection limits are $L/240$ or $L/360$, depending on load types, for beams supporting elevators the maximum allowable deflection is considered $L/1666$ (Tornquist, 2014). This led to the need to increase the beam size beyond that needed for bending strength due to the need for a greatly increased major axis moment of inertia $I_x$ value.

![Minimum Rated Load for Passenger Elevators](image)

*Figure 10: Safety Code for Elevators and Escalators, ASME A17.1-2007 pg 242*
7.0 Design of Academic Facility

The major focus of this project was the structural design of the new buildings to be erected on the Salisbury Estates property. This chapter details the structural designs the team completed for this part of the project.

7.1 Roof/Floor Beam and Girder Design

In order to understand where to begin for the sizing of members, *The Architect’s Studio Companion* (Allen, 2005) was referred to for typical member sizes based on system characteristics. The design was carried out from the top-down, directing attention to the roof first. Load Resistance Factor Design (LRFD) was used to design the members, and an Excel spreadsheet was designed to assist in this iterative process for most members. A copy of the Excel spreadsheet used and hand calculations showing specific steps during the design process can be found in Appendix G.

A truss system was considered initially due to it being a more structurally efficient design than a beam-and-girder system. Referring to Figure 11 – the Structural Steel Truss) guidelines within the *Companion* – exposed the need for deep truss systems in order to span the large buildings proposed. This would cause issues with the zonal height restrictions discovered in preliminary research; therefore, it was decided to design the roof with beams and girders similar to the floors.

![Structural Steel Trusses](image)

*Figure 11: Typical Span-based Depth for Steel Trusses, The Architect’s Studio Companion*
The approach for design of the floor members was considered next. The team referred to Figure 12 specifying typical span ranges for different types of structural systems. This provided a basis for the limitations on span lengths, which aided in the placement of columns and connecting elements in the systems.

<table>
<thead>
<tr>
<th>STRUCTURAL SYSTEM</th>
<th>Pages</th>
<th>Span Range</th>
</tr>
</thead>
<tbody>
<tr>
<td>WOOD</td>
<td></td>
<td>3 m</td>
</tr>
<tr>
<td>Joists</td>
<td>60–61</td>
<td>10'</td>
</tr>
<tr>
<td>Decking</td>
<td>68–69</td>
<td>20'</td>
</tr>
<tr>
<td>Solid Beams</td>
<td>70–71</td>
<td>30'</td>
</tr>
<tr>
<td>Rafter Pairs</td>
<td>62–63</td>
<td>40'</td>
</tr>
<tr>
<td>Light Floor Trusses</td>
<td>64–65</td>
<td>50'</td>
</tr>
<tr>
<td>Light Roof Trusses</td>
<td>64–65</td>
<td>60'</td>
</tr>
<tr>
<td>Glue-Laminated Beams</td>
<td>72–73</td>
<td>70'</td>
</tr>
<tr>
<td>Heavy Trusses</td>
<td>74–75</td>
<td>80'</td>
</tr>
<tr>
<td>Glue-Laminated Arches</td>
<td>76–77</td>
<td>90'</td>
</tr>
<tr>
<td>Domes</td>
<td></td>
<td>100'</td>
</tr>
<tr>
<td>BRICK &amp; CONCRETE MASONRY</td>
<td></td>
<td>150'</td>
</tr>
<tr>
<td>Lintels</td>
<td>84, 90</td>
<td></td>
</tr>
<tr>
<td>Arches</td>
<td>85</td>
<td></td>
</tr>
<tr>
<td>STEEL</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Corrugated Decking</td>
<td>100–101</td>
<td></td>
</tr>
<tr>
<td>Lightweight Steel Joists</td>
<td>94–95</td>
<td></td>
</tr>
<tr>
<td>Beams</td>
<td>102–103</td>
<td></td>
</tr>
<tr>
<td>Open-Web Joists</td>
<td>104</td>
<td></td>
</tr>
<tr>
<td>Single-Story Rigid Frames</td>
<td>105</td>
<td></td>
</tr>
<tr>
<td>Heavy Trusses</td>
<td>106</td>
<td></td>
</tr>
<tr>
<td>Arches and Vaults</td>
<td></td>
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<tr>
<td>Space Frame</td>
<td></td>
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<tr>
<td>Domes</td>
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<td>Cable-Stayed</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Suspension</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

Figure 12: Span Ranges for Structural Systems, The Architect’s Studio Companion

The greatest difficulty when designing the framing plans came in the bays containing staircases and elevators. In these bays the beam spacings and layouts had to be adapted so as not to interfere with the stairs and elevators. A boxing in approach to the stairwells was decided upon after consulting framing plans from Gateway as shown in Figure 13.
Doing this raised the need to adapt the Excel spreadsheets used in the assistance of the member sizing to reflect the loading differences. While the elevators themselves did not cause the need to adjust any framing plans, they were looked at carefully and determined to not be in the path of any desired beam locations. Bays with staircases however required an adjustment of beam location. These adapted framing plans can be seen in Figures 14 and 15.
During design it was decided to cope the beam flanges for necessary connections. One final consideration when choosing the final member sizes was the debate between efficiency versus ease of construction. There were many different size members that were deemed usable in
certain areas of the building. In cases where the beam sizes changed frequently it was decided to size up some of the members in order to provide a more repetitive construction. This could also lower costs in some cases, even though more steel is being used, because there are less uniquely sized members being ordered. However, instances where there were many of the same size beam before a change it was determined not to size up the smaller members due to great increase in weight and lack of cost benefit.

7.2 Columns

Following the design of the beams and girders, the sizing of columns was addressed. Prior to the start of calculations, design considerations about the loading acting on the column had to be confronted. These considerations consisted of two primary components: the selection of the governing loading combination and the selection of the specific column to be used for design. Various loading cases from ASCE 7 were evaluated to determine which combination resulted in the greatest load. Additionally, due to the inconsistent column placement throughout the building, eight different bays consisting of varying tributary areas were investigated. Design loads acting on each bay were conservatively deemed consistent for ease of calculations, however bays along the exterior of the structure included the weight of the exterior enclosure. It was determined that Bay D, shown in Figure 16, with the largest tributary area resulted in the largest axial load, despite the lack of exterior enclosure and its added weight.

![Figure 16: Column Bays Investigated for Design](image)

Column sizing was carried out for the column located on the first floor of Bay D. The first-floor column within the largest bay was chosen to represent the highest amount of loading acting on a column anywhere within the structure. Once sized, it was decided that the selected section was to be used for every column within the entire building for ease of construction.
Design calculations and an accompanying Excel document utilized in design can be found in Appendix H.

7.3 Design of the Lateral-Load Resisting System

In order to design the lateral-load resisting systems of the structure, both seismic and wind loads were addressed. The seismic design rating was initiated. Beginning with the determination of the mapped spectral acceleration for short periods (Ss) and the mapped spectral accelerations for 1-second intervals (S1) values for the City of Worcester. The Ss and S1 values for all towns and cities in Massachusetts can be found in the 9th Edition Amendment to the Massachusetts State Building Code. The seismic design categories and site classifications were defined based on equations found in ASCE 7, from there, the risk category was determined as risk category IV, site class C, which is the most detrimental scenario of seismic activity that the Salisbury Estates geographical location can be exposed to.

Lateral reinforcement was designed for the corner bays of the structure as they are composed of the largest N-S and E-W members and will brace the largest deflection points. The N-S face of the structure was addressed first. Design for seismic loading was carried out initially. Data that was determined through the methods described above were input into a seismic base shear and vertical shear distribution Excel spreadsheet formed based on ASCE 7 requirements, seen in Appendix I, that was used to organize and evaluate seismic base shear and vertical shear distribution. The expected seismic forces for each level of the structure were identified and input into a RISA 2D model representing the three-story steel frame. Images of the final RISA 2D model can be found in Appendix L. The thought process used was that the psf force values calculated using the Excel document could be applied to applicable areas of the wall faces of the structure.

The team tested the loading case that resulted in the greatest stresses and deflections on the structure. This same process of evaluating the resulting deflections was executed for wind loads to determine whether seismic or wind would be the governing lateral force condition. The accompanying Excel document used to aid in the determination of wind loads can be found in Appendix J. Greater deflections were noticed under seismic loading conditions, therefore the bracing was designed for these cases. Bracing was then added and iteratively adjusted until deflections were within tolerable values according to the drift analysis Excel document as seen in Appendix K. This same process was carried out for the E-W section. The final bracing member sizes can be seen in Table 9.
<table>
<thead>
<tr>
<th>Structure Face/Section</th>
<th>Lateral Reinforcement Member Size</th>
</tr>
</thead>
<tbody>
<tr>
<td>N-S (35-foot) section</td>
<td>W12x14</td>
</tr>
<tr>
<td>E-W (46-foot) section</td>
<td>W10x12</td>
</tr>
</tbody>
</table>

### 7.4 Footings

The first step in designing the footings for the foundations system was to design the steel baseplates to transfer loads from the structural steel columns to the supporting concrete elements. In order to determine how much force was to be supported, the column axial loads had to be addressed. These were determined by referencing the Excel spreadsheet used for column design and identifying the greatest resultant vertical axial force. This was simplified since the columns were designed to be the same size (W12x72) throughout the structural system. Since the proposed building did not include the design of a basement level, it was decided that the use of pedestals between the baseplates and footings was unnecessary.

The area of the baseplates had to be designed to assure the concrete footing can withstand the forces being transmitted through the baseplate. Bearing pressure for normal weight concrete was used in these calculations as that will be used for the footings. Once the baseplate area required based on this maximum bearing pressure was determined, dimensions were chosen to meet this requirement while minimizing and normalizing the moments created by them. Moments resulting from the pressure on the edges of the baseplate determine the thickness required for the baseplates. For this reason minimizing the moments will result in a thinner baseplate and a lower-cost structure.

The connections between the baseplates and footings then had to be designed. To determine the required area of the connecting bolts, 0.5% of the baseplate area was calculated, based on the parameters of the *Design of Concrete Structures* (Darwin, 2016). Due to OSHA requirements there will be four anchoring bolts, one in each corner. Therefore, required area of bolts was divided by the minimum of four bolts to determine bolt sizes. From here, Table J3.4 of the AISC Steel Manual was referenced to identify the minimum required spacing between the center of the connecting bolts and edge of baseplate.

The final component to design was footings. Based on column placement throughout the framing system, the maximum footing size allowed to avoid overlap 30’x22’. To determine the area of footing actually required, the soil bearing pressure of the Salisbury Estates site had to be assessed. The soil type was found to be silty sand with a corresponding soil bearing capacity of 3000 psf (NRCS) (Concrete Network, 2015). Assuming a footing depth of five ft, the effective
bearing capacity then had to be addressed taking into account the depth and density of the concrete. Required footing area was then determined using the loads and effective bearing capacity. With the baseplate and footing dimensions determined the footing’s effective depth was calculated using the nominal punching-shear strength along with the factored shear force. After ensuring that the footing withstands beam-shear and bearing, the bending moment was used to determine the reinforcement required by for the footings. Final sizes and reinforcement values can be seen in Table 10. Due to dimensional constraints from the bar diameters, required spacing, steel cover width, and concrete casing the number of reinforcing bars was increased to allow for a uniform distribution. Detailed drawings of the baseplate and footing designs, accompanied by their calculations, can be located in Appendix M.

Table 10: Baseplate and Footing Dimensions and Reinforcement

<table>
<thead>
<tr>
<th></th>
<th>Base Plate</th>
<th>Footing</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>Size</strong></td>
<td>16”x12”x1.5”</td>
<td>20’x20’x5”*(1)</td>
</tr>
<tr>
<td><strong>Reinforcement</strong></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Size</td>
<td>No. 5</td>
<td>No. 18</td>
</tr>
<tr>
<td>Quantity</td>
<td>4</td>
<td>192 (6 layers of 32)</td>
</tr>
<tr>
<td>Spacing</td>
<td>1” in from corners</td>
<td>5.25” between 1” between layers</td>
</tr>
</tbody>
</table>

*(1)*: 5 foot thickness was based on a conservative assumption
8.0 Cost Analysis

Although the structural design was the focus of the project, assessing cost was an important addition. Cost estimates were calculated for both the demolition of the existing Salisbury Estates and the construction of the new proposed facilities. All estimates for cost were executed using data taken from RSMeans; specifically the *RSMeans Building Construction Costs* data and the *RSMeans Square Foot Costs* data (Gordian, 2018).

8.1 Demolition of Existing Site

The cost of developing the Salisbury Estates complex involves not only the construction of the facilities, but also the demolition of the existing site beforehand. The team was supplied with information on the existing Salisbury Estates by The WPI Facilities Department. Through referencing these documents, it was found that the complex consists of 108 units, each about 850 square feet. Using an assumed average height of 25 feet to each unit (gathered from previously visiting the site), a cost per cubic foot of space was taken from *RSMeans Building Construction Costs* data to estimate the demolition cost of the site to be about $800,000 (Gordian, 2018).

8.2 Construction of New Site

A cost estimate for the designed structures was developed in three parts. First, an estimate for the structural system of the building was developed by calculating the cost based on unit cost values per linear footage of each member section in the building. Certain member sizes used within the building were not listed in the RSMeans data. When this was the case, the next largest section listed in the data was selected and costs for the larger section were used in place of the missing data. Specific members for which this occurred are listed in Table 11. A comprehensive list of all section sizes and associated costs can be found in Appendix N.
The sprinkler system cost estimate was calculated on a square foot basis at $3.00 per square foot for brand new construction. The fire alarm system cost estimate was calculated on a square foot basis at $2.00 per square foot for brand new construction (Tyco, 2017). The fire protection costs were determined for both the academic and residential buildings based on the same cost per square foot basis. Table 12 shows this cost breakdown.
Table 12: Fire Protection Costs

<table>
<thead>
<tr>
<th></th>
<th>Academic Building</th>
<th>Residential Building</th>
</tr>
</thead>
<tbody>
<tr>
<td>Total Square Footage</td>
<td>75000</td>
<td>104100</td>
</tr>
<tr>
<td>Sprinkler system cost</td>
<td>$225,000.00</td>
<td>$312,300.00</td>
</tr>
<tr>
<td>Fire alarm cost</td>
<td>$150,000.00</td>
<td>$208,200.00</td>
</tr>
</tbody>
</table>

Following the cost estimate for the structural system and fire protection system, costs for various finishes to the building were calculated through the use of RSMeans Square Foot Costs data. A list of all non-structural specific line items that were investigated can be found in Appendix O.

8.3 Final Cost

After summing both structural and non-structural elements of the academic building, two final costs for the construction of the building were found. Throughout calculations, costs for each element were found for the cost of material plus labor. In addition to this, the cost of overhead and profit that the contractors would likely charge for their services was added in to the total coast for a second estimate. For each of these estimates, a cost per square foot was calculated as well. Table 13 shows these final cost estimates.

Table 13: Total Academic Building Cost Estimate

<table>
<thead>
<tr>
<th></th>
<th>Total Cost</th>
<th>Total Cost + O&amp;P</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>$19,550,151</td>
<td>$19,953,118</td>
</tr>
<tr>
<td></td>
<td>$260.67 per S.F.</td>
<td>$266.04 per S.F.</td>
</tr>
</tbody>
</table>
9.0 Conclusions and Recommendations

This section will discuss what was completed throughout the course of this project in order to satisfy the team’s capstone design requirements. This will include the process to go from our initial ideas of the project, through the research required, and to the final design, fire protection and life-safety analysis, and cost estimations. Following this conclusion will be recommendations on what could have been done differently for the project along with what future teams could look at for a continuation of our project.

9.1 Academic Building Conclusion

Upon completion of preliminary research and site layout design, the team moved forward with the floor layout design, structural design (using LRFD design), fire protection and life safety design, and cost estimation for the academic building. The floor layout design was finished first as it allowed for an accurate and desirable placement of columns to minimize interference within the floor space. Using beam and girder sizing design aids, the allowable distance separating the columns was determined and member lengths were chosen.

The life safety analysis was conducted while the architectural drawings were being finalized to ensure there was sufficient egress space from the building as well as proper door swing with respect to occupant load in each room of the building. Following this, fire protection plans were made in the form of AutoCAD plans for the installation of a fully automatic sprinkler system.

Once the bay sizes were determined, ASCE 7 was used to determine design loads for sizing of members. The team took a top-down approach to size members, starting with the roof bays, followed by the second and third floors, and ending with the first floor. With beams and girders sized and self-weights determined, the columns supporting these bays were then designed. The final part of developing the framing plan was the design of lateral bracing in the corner bays. With the framing designs completed, the team moved to foundation design. Due to the lack of a basement in this building, the foundation design involved simple baseplates and footings for each column. While the team completed the structural design of member sizes for this building, there was not enough time to design all of the system connections. Given more time, the team would have carried out typical connection design for the academic building.

In order to develop an accurate cost estimate for the building, cost data from RSMeans publications was used along with estimated fire protection costs. Following the completion of the structural design, cost data from RSMeans publications was consulted for costs per linear foot of all structural members. In addition to these linear foot values, all nonstructural elements were estimated with cost per square foot values. The final cost added was a standard cost per square foot value for the sprinkler system.
9.2 Residential Building Extrapolation

The team used their findings from the design of the academic structure to extrapolate an estimated cost for the construction of the residential building. The extrapolation of cost was calculated by multiplying the cost per square foot of the academic building by the total square footage of the residential building. Table 14 shows these estimates.

<table>
<thead>
<tr>
<th>Total Cost</th>
<th>Total Cost + O&amp;P</th>
</tr>
</thead>
<tbody>
<tr>
<td>$27,135,747</td>
<td>$27,694,764</td>
</tr>
</tbody>
</table>

It should be noted that these cost estimates are based off the calculations of an academic facility and adjustments should be made to these figures to reflect a more accurate cost of a residential facility. Within the construction of a residential space, certain costs exist that would not be present in an academic setting. Included in these are additional plumbing fixtures such as shower stalls and a greater number of toilets to meet increased demand. Furthermore, added security features would likely be included in the construction of a residential facility such as optical sensors in the hallways. Additionally, in the preliminary design for the residential building, there was the inclusion of a dining facility located on the first floor of the structure. This facility would likely raise the overall cost of the proposed residential building.

The elements included in the final cost for each building are structural, life safety, and occupancy code necessities. The cost estimate does not include furnishings for either building. It should be noted that due to the difference in use of each of these buildings, costs associated with these furnishings would likely create a further disparity in cost.

9.3 Recommendations

Based on the team’s finishing point for this project, it is recommended that a future team continue this project further focusing on the aspects to be discussed herein. A future team should spend the time to research the connection design for the framing in the residential building. With framing connections designed, the team could then go forward to complete the structural and fire protection design of the residential building and not have to rely on extrapolation. Aside from the structural design of the buildings, there could be more in-depth fire protection design throughout. The main focus for the fire protection was the sprinkler systems and egress design requirements. A future team could go further with this by completing life safety plans including emergency light, exit signage, evacuation plans, fire alarm drawings (including location of smoke detectors), horn/strobe devices, manual fire alarm pull station locations, and fire extinguisher placement. The last recommendation is to develop a LEED certification plan for the buildings’ construction and fixtures. Sustainable development is becoming a very important aspect for new structures in
today’s industry. WPI has also focused on this for its new buildings, so this could be a relevant aspect to look more into.
10.0 Bibliography


*City of Worcester Zoning Ordinance 2018 Edition*


“FEMA Flood Map Service Center” FEMA, msc.fema.gov/.


*Massachusetts State Building Code (MSBC) 9th Edition*


MEANS., R. S. *Square Foot Costs with RSMeans Data*. Gordian, 2018.

*National Fire Protection Association NFPA 13 2013 Edition*
National Fire Protection Association NFPA 72 2013 Edition
National Fire Protection Association NFPA 80 2013 Edition
National Fire Protection Association NFPA 70 2014 Edition
National Fire Protection Association NFPA 221 2015 Edition


Appendix A: Final Proposal

Redesign of Salisbury Estates

Major Qualifying Project Proposal

Presented by:
Zachary Abbott, Mark DellaCroce, Dylan Felty, & Tyler Kornacki

Presented to:
Professor Leonard Albano and the Worcester Polytechnic Institute Civil Engineering Department in partial fulfillment of the requirements of the Degree of Bachelor of Science 2018-2019
Abstract

With the Worcester Polytechnic Institute (WPI) undergraduate student body growing every year, there is a need for expansion of on-campus space. The apartment complex of Salisbury Estates serves as an outlet to help provide this necessary accommodation, but the poor conditions of the site have become recognized more-so, presenting the need for development of the site. Through interviews with WPI staff and preliminary research on site restrictions and design criteria, alternatives will be assessed and a layout will be decided upon. The floor plan and framing plan composing the proposed facilities will then be developed and structural design and analysis of the required beams, columns, and footings will be performed in accordance with relevant specifications. A life safety analysis addressing egress will also be performed before a cost analysis is executed to estimate demolition, construction, and labor costs. Deliverables will include the structural calculations carried out for design, models of plans and renderings, and a cost estimate, all of which will be presented with a final report and poster.
1. Introduction:

The following proposal is derived from what Worcester Polytechnic Institute (WPI) lacks most in terms of infrastructure on campus. This proposal offers a solution to the current housing and classroom space shortages that will only become more of a concern as incoming class sizes continue to increase. The comprehensive redesign of the Salisbury Estates property will involve the demolition of all existing buildings, roadways, paths, and landscape to allow for a new complex of large residential and academic buildings as well as an additional dining facility. A report will include a digital model of the proposed buildings, a site plan for the entire site, and a full cost estimate. This will also demonstrate the significant addition of classrooms, common areas, dormitories, and parking spaces provided to accommodate the growing undergraduate population. Areas of depth will include structural design and analysis as well as life safety and fire protection, all of which will be addressed in accordance with applicable codes and standards.

The residential building design will include meeting spaces and large common areas as well as tech suites. These functional spaces will promote collaboration and increase the appeal of the complex. The design will incorporate housing for several hundred students within a two resident per unit style dormitory facility. The first floor will also include a dining area and a connecting lounge area available to WPI members. The separate academic building design will include a mixture of lecture halls, classrooms, and offices in order to appeal to the several needs of the institution and create a greater draw to the development.

More parking will be required to accommodate the new users of the facilities. It will be undesirable to erect an entire parking garage for this purpose. Therefore, the parking will be kept similar to its current style with street and lot parking. Salisbury Estates occupies a substantial amount of land, some of which will remain available for development outside of the building construction. A balance will be determined regarding what is developed into parking and what is left alone so as to form an open, green space that connects the facilities similar to the Quadrangle on campus currently. In addition to providing students a safe place to go outside and enjoy leisurely activities, this open space will also provide opportunity for further development in the future if necessary.
2.0 Background:

Several factors must be considered for the design of a new structure. The current site will be assessed to determine property lines and land conditions. The design will be developed with input from WPI faculty and staff to ensure that the wants and needs of the University are considered. Code research will be conducted to ensure that the proposed buildings will be designed in accordance with applicable codes and standards. The project’s impact on the community will be considered to ensure that the new development will fit well into the area and not cause issues with surrounding properties. Designs are only accepted if they are economically feasible; therefore, the cost of the proposed development will be assessed. Ethics is also a topic to be constantly aware of in order to ensure that items are being addressed in a correct, professional manner throughout the project.

2.1 Current Site Information

The proposed buildings will be situated within the lot of WPI’s Salisbury Estates. This area is located along Massachusetts Route 122A (Park Avenue), between Salisbury Pond and the Worcester Center For Crafts as seen in Figure 1 below. For students to access this complex by foot, they must either walk along an indirect sidewalk or an unpaved and unlit path along Salisbury Pond. Alternatively, access by car is only possible through a single entrance and exit on Park Avenue (Rumford Avenue). The plot of land is quite expansive and has primarily level topography. According to flood maps from the FEMA Flood Map Service Center, this land is in an area of minimal flood risk despite its close proximity to Salisbury Pond; therefore, flooding should not be an issue.
In addition to the plot of land Salisbury Estates is currently on, WPI owns some additional neighboring property. The large building located between the Worcester Center for Crafts and Avis Car Rental belongs to WPI and is currently being used to store equipment for WPI Facilities. Aside from this building, all of the land between the Worcester Center for Crafts and Grove Street is under WPI control. This includes a small parking area to the north that could be expanded southward to provide additional parking and a through way for an extra access point to the property.

2.3 Design Parameters

The following sections demonstrate the technical aspects of our final building designs and what must be incorporated with respect to fire code requirements and occupancy classifications. The limiting factor for the design of the building lies in the codes, standards, and
ordinances put forth by the City of Worcester and the Commonwealth of Massachusetts. The building is to be designed according to the 2015 Edition of the International Building Code (IBC), the 9th edition of the Massachusetts State Building Code (MSBC), all National Fire Protection Association (NFPA) codes and their respective editions adopted by Massachusetts, and all zoning and ordinances for Worcester.

2.3.1 Zoning Requirements

Across Massachusetts, cities and towns are divided into different zoning districts that serve to regulate the use of specific plots of land and govern building characteristics across different neighborhoods and usage areas. Although the entirety of WPI’s main campus is currently zoned as Institutional (IN-S), the Salisbury Estates property is currently zoned as Limited Residential (RL-7) which presents several design restrictions that do not apply to WPI’s campus on the hill. Of significant relevance to this project are the restrictions on permitted land use, permitted dimensions, and off-street accessory parking requirements. These regulations can all be found within the City of Worcester Zoning Ordinance in Tables 4.1, 4.2, and 4.4 respectively. Table 1 below shows the major considerations associated with each regulation that were taken into account during the preliminary discussion of design alternatives.

<table>
<thead>
<tr>
<th>Ordinance Table Number</th>
<th>Regulation</th>
<th>Consideration</th>
</tr>
</thead>
<tbody>
<tr>
<td>4.1</td>
<td>Permitted Uses by Zoning District</td>
<td>● Dormitory Space: Permitted under “Special Permit”</td>
</tr>
<tr>
<td></td>
<td></td>
<td>● Schools Non-Profit: Permitted</td>
</tr>
<tr>
<td>4.2</td>
<td>Permitted Dimensions by Zoning District</td>
<td>● Maximum of three stories</td>
</tr>
<tr>
<td></td>
<td></td>
<td>● Maximum overall height of 45 feet</td>
</tr>
<tr>
<td>4.4</td>
<td>Off-Street Accessory Parking Requirements</td>
<td>● 0.33 parking spaces required per dwelling unit</td>
</tr>
<tr>
<td></td>
<td></td>
<td>● Ten parking spaces required per classroom</td>
</tr>
</tbody>
</table>

Referring to these regulations impacted the scope of alternative designs discussed by the team. Initially, the design was to incorporate a building height of five stories; however, the
Worcester Ordinances identified that the maximum height requirement for the specified zone was only three stories. This research also provided a clearer picture of how much parking will be required based on the number and types of occupants.

Further investigation revealed that there exists a 100-foot buffer zone from the waterline of Institute Pond which means that all new construction must be set back 100 feet from the pond. If construction is desired beyond the 100-foot buffer zone, then a variance must be requested to receive permission.

2.3.2 Occupancy Classification for Residential Building and Academic Building

The team has decided to propose a three-story residential hall that will include bedrooms, common spaces, and tech suites. A dining facility will also be implemented on the 1st floor connecting the two halls in the center and extending beyond the back of the building to include a full commercial kitchen. The academic building design will incorporate offices, tech suites, classrooms, and lecture halls. The table below shows the code requirements and restrictions that must be taken into account of the proposed building designs. These occupancy classifications (shown in Table 2 below) will be important to determine the final occupant load of every space in the report of this project after final areas of the building are discussed and agreed upon.

Table 2: Design Parameters Code Implications

<table>
<thead>
<tr>
<th>Occupancy in Question</th>
<th>Code Requirement</th>
<th>Code Reference</th>
</tr>
</thead>
<tbody>
<tr>
<td>Residential Hall Occupancy Classification</td>
<td>Residential Group R-2: sleeping spaces of more than two dwelling units where occupants are primarily in nature, which includes dormitories.</td>
<td>MSBC Section 310.4.</td>
</tr>
<tr>
<td>Occupancy Separation</td>
<td>Group R-2 occupancies shall be separated from other accessory occupancies.</td>
<td>MSBC Section 508.2.4</td>
</tr>
<tr>
<td>Need for Automatic Sprinkler System</td>
<td>Group R occupancies shall be equipped throughout with an automatic sprinkler system.</td>
<td>MSBC Section [F] 420.5</td>
</tr>
<tr>
<td>Occupancy Classification</td>
<td>Description</td>
<td>Code Section</td>
</tr>
<tr>
<td>--------------------------</td>
<td>-------------</td>
<td>--------------</td>
</tr>
<tr>
<td>Entry common space</td>
<td>Assembly Group A, a portion of a building used for gathering of persons for purposes including recreation shall be classified as Assembly.</td>
<td>MSBC Section 303.1</td>
</tr>
<tr>
<td>Dining hall occupancy</td>
<td>Assembly Group A-2, use of cafeterias and similar dining facilities.</td>
<td>MSBC Section 303.3</td>
</tr>
<tr>
<td>Commercial kitchen</td>
<td>Assembly Group A-2, associated commercial kitchens attached to a dining facility.</td>
<td>MSBC Section 303.3</td>
</tr>
<tr>
<td>Office</td>
<td>Business Group B Occupancy</td>
<td>MSBC Section 302</td>
</tr>
<tr>
<td>Tech Suites</td>
<td>Group B Occupancy because they will be intended to hold less than 50 people</td>
<td>MSBC Section 302</td>
</tr>
<tr>
<td>Classrooms and lecture</td>
<td>Assembly Group A</td>
<td>MSBC Section 303.1</td>
</tr>
</tbody>
</table>

The residential spaces shall remain separate from the main entryway with the attached dining facility; furthermore, for security reasons, there must be double door vestibules with key card access from the ingress side of the building entryway to the residential areas. This is to prevent unwanted persons from entering the residential halls. The occupancy classifications will be used to calculate the occupant loads which can be present at any given time in the residential space, entryway common space, dining space, and commercial kitchen space.
2.3.2.1 Building Construction Type

The residential building can be designed with Type I or Type II construction which will ideally include a full steel structural frame with brick facade. The construction type and occupancy classifications can then be used to calculate the maximum allowable building footprint per the MSBC. Worcester Ordinances have stricter restrictions than those found in the MSBC; therefore, the Worcester Ordinances take precedence. Type I construction is defined by the use of steel, and Type II construction is defined by the use of brick or masonry. Types III, IV, and V construction includes the use of combustible materials and are therefore undesirable for use in a residential dormitory building.

2.3.2.2 Building Size Limitations

The table below shows the building size limits with respect to construction type. Since the buildings will be comprised of mainly steel structural elements, the steel is required to have a certain level of fire resistance in an hour rating system. Table 3 below shows the requirements per the MSBC; however, as previously stated, Worcester Zoning Ordinances take precedence over the MSBC.

Table 3: Design Parameters Construction Considerations

<table>
<thead>
<tr>
<th>Occupancy in Question</th>
<th>Code Requirement</th>
<th>Code Reference</th>
</tr>
</thead>
<tbody>
<tr>
<td>Building Height allowance</td>
<td>Unlimited for residential occupancies of Type I construction when equipped with an automatic sprinkler system</td>
<td>MSBC Table 504.3</td>
</tr>
<tr>
<td>Stories permissible</td>
<td>Unlimited under Type I construction when equipped with an automatic sprinkler system</td>
<td>MSBC Table 504.4</td>
</tr>
<tr>
<td>Building area allowance</td>
<td>Unlimited under residential occupancies of Type I construction provided the building is equipped with an automatic sprinkler system</td>
<td>MSBC Table 506.2</td>
</tr>
<tr>
<td>Type IA structural elements</td>
<td>Three-hour fire resistance rating for all structural members of the building frame</td>
<td>MSBC Table 601</td>
</tr>
<tr>
<td>-----------------------------</td>
<td>---------------------------------------------------------------------------------</td>
<td>-----------------</td>
</tr>
<tr>
<td>Bearing walls</td>
<td>Shall have a three-hour fire resistance rating</td>
<td>MSBC Table 601</td>
</tr>
<tr>
<td>Roof Structure</td>
<td>Shall have a one-and-a-half-hour fire resistance</td>
<td>MSBC Table 601</td>
</tr>
</tbody>
</table>

2.3.2 Fire Requirements with Respect to Site Plan

The following sections contain the limits to the site plan design with respect to fire requirements including the fire access road, the location of the entrances to the buildings, and the fire department connections to the buildings. These requirements will assist in shaping the landscape of the site plan and identifying where parking spaces will be located.

2.3.2.1 Fire Department Connections

The design of both the residential and academic buildings must take into account the location of fire department connections. MSBC Section [F] 912: Fire Department Connections states that with respect to hydrants, driveways, buildings and landscaping, these fire department connections shall be located so that fire apparatus and hoses connected to supply the system will not obstruct access to the buildings for other fire apparatus. The fire department connections shall be located on the street side of buildings, fully visible and recognizable from the street or nearest point of fire department vehicle access. The section continues to acknowledge that the fire department connections shall be maintained at all times and shall never be obstructed by fences, bushes, trees, walls or any other fixed or moveable object.

2.3.2.2 Fire Access Road

According to MSBC Section 503.1.1, approved fire apparatus access roads shall be provided for every facility, building, or portion of a building hereafter constructed or moved into or within the jurisdiction of the project. The section further explains that the fire access road shall extend to within 150 feet of all portions of the facilities and all portions of the exterior walls of the first stories as measured by an approved route around the exterior of the buildings. It is also noted that the fire access road shall have an unobstructed width of no less than 20 feet, and an unobstructed vertical clearance of no less than 13 feet-6 inches.
2.3.3 Fire Requirements with Respect to Building Design

The following table addresses design codes and specifications regarding occupancy loading, egress plans, sprinkler systems, as well as the materials and furnishings that the team will have to abide by in designing specific components of the buildings.

Table 4: Design Parameters for Life Safety Considerations

<table>
<thead>
<tr>
<th>Category of Consideration</th>
<th>Code Requirement</th>
<th>Code Reference</th>
</tr>
</thead>
<tbody>
<tr>
<td>Occupancy Load Calculations</td>
<td>Occupant load placards shall be provided for each space of the buildings.</td>
<td>MSBC 1607.1</td>
</tr>
<tr>
<td>Egress Plan</td>
<td>Evacuation plans shall be provided for each level of the buildings demonstrating primary and secondary means of egress</td>
<td>NFPA 101 2013 Edition</td>
</tr>
<tr>
<td>Sprinkler Design</td>
<td>Building shall be sprinklered throughout</td>
<td>NFPA 13</td>
</tr>
<tr>
<td>Building Entrance Access</td>
<td>Building entrance access shall be within certain distance of the fire access road, through which the main fire alarm panel shall be accessible</td>
<td>MSBC 504.1</td>
</tr>
</tbody>
</table>

2.4 Community Impact

The redesign of the Salisbury Estates property will affect the surrounding Worcester community. From the demolition of the existing property to the active construction of the buildings, the proposed buildings will impact not only the WPI community but also the community of Worcester.

2.4.1 Impact on the WPI Community

With student populations at WPI growing every year, on-campus residential space has reached its capacity; all available rooms have been filled and some students are even on waitlists.
Departments across the institution agree that additional housing for undergraduates is the top priority moving forward. Without access to on-campus housing, students are forced to move to off-campus alternatives which can vary in quality and safety. In extreme cases, some students may even resort to lengthy or inconvenient commutes from neighboring communities if they are unable to secure housing on campus.

Although residential space has been identified as the top priority for future development, the need for additional academic space is also present. As the size of the student body grows, so does the amount of classes offered. With only so many hours in a day, WPI has begun to encounter scheduling difficulties due to a lack of available classrooms for professors to instruct courses. The growing student body will only intensify this issue in coming years if additional classroom space is not added to campus.

The new and improved Salisbury Estates property will offer a solution to this lack of adequate residential housing and classroom space at WPI.

### 2.4.2 Impact on the Greater Worcester Community

The new construction on Salisbury Estates has significant potential to impact the greater Worcester community. In order to begin the construction for the project, all existing structures must be demolished and landscape cleared. From this initial step, this project will impact the greater Worcester area. With demolition and new construction comes loud noise, displacement of existing residents, and potential impediment of public utilities for surrounding buildings, all of which will affect residents of the community for the duration of the project. Additionally, once built, the new structures will themselves become part of the community. The structures will be designed to fit into the theme of the surrounding area and not stick out as loud additions to the pre-existing neighborhood.

### 2.5 Sustainability

Projects aiming to improve infrastructure must be effective not only today but also in the future. In order to accomplish this, sustainability must be addressed in design. The proposed redesign of Salisbury Estates will need to be environmentally friendly and sustainable to accommodate for future generations as the WPI community continues to grow. It is important to assess the conditions of the site itself and design so as to minimize the project’s impact on the plot of land due to environmental concerns such as flooding and erosion. The team will need to be conscious of the building materials and construction processes used along with the design itself.
2.6 Ethics

Throughout this project several ethical considerations must be kept in mind. The American Society of Civil Engineers (ASCE) says that “Ethics is integral to all decisions, designs, and services performed by civil engineers.” There are ethical specifications that must be addressed for every project: designing the project in the best interest of the client, being truthful in the cost and timeline for the project, and not using substandard materials or techniques to save money. In addition to these, the team must address the current Worcester residents of Salisbury Estates that will have to be relocated upon construction. These residents will be alerted early in the planning process to provide them with ample time to find a new residence; assistance for these residents will also be provided by WPI. By adhering to these procedures, in addition to ASCE’s assertion that “engineers uphold and advance the integrity, honor, and dignity of the engineering profession by using their knowledge and skill for the enhancement of human welfare and the environment, being honest and impartial and serving with fidelity the public, their employers and clients, striving to increase the competence and prestige of the engineering profession, and supporting the professional and technical societies of their disciplines” (ASCE, 2017), this project will be completed ethically and appropriately.
### 3.0 Methodology

*Table 5: Methodology Breakdown*

<table>
<thead>
<tr>
<th>Project Methodology Summary</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>Establish Design Goals</strong></td>
</tr>
<tr>
<td>Conduct interviews with Residential Services and Facilities</td>
</tr>
<tr>
<td>Investigate site layout options and evaluate alternatives</td>
</tr>
<tr>
<td><strong>Structural Analysis and Design of Residential Facility, Dining Facility, and Academic Facility</strong></td>
</tr>
<tr>
<td>Determine required use of space and develop floor plan</td>
</tr>
<tr>
<td>Develop a framing plan</td>
</tr>
<tr>
<td>Perform design calculations for structural system members including beams, columns, and footings</td>
</tr>
<tr>
<td><strong>Life Safety Analysis</strong></td>
</tr>
<tr>
<td>Configure egress plan</td>
</tr>
<tr>
<td>Develop architectural drawings</td>
</tr>
<tr>
<td><strong>Cost Analysis</strong></td>
</tr>
<tr>
<td>Estimate demolition costs</td>
</tr>
<tr>
<td>Estimate total material quantities and associated costs</td>
</tr>
<tr>
<td>Estimate architectural and construction labor costs</td>
</tr>
<tr>
<td><strong>Deliverables</strong></td>
</tr>
<tr>
<td>Final report and poster</td>
</tr>
<tr>
<td>Structural calculations</td>
</tr>
<tr>
<td>Computer models including Revit renderings and AutoCAD floor plans and cross sections</td>
</tr>
<tr>
<td>Total project cost estimate</td>
</tr>
</tbody>
</table>

### 3.1 Establish Design Goals and Develop Site Plan

Although the redesign of Salisbury Estates is the objective of the team, it was agreed that the perspectives of the school’s staff should provide insight towards the actual wants and needs regarding the development of this complex and contribute to a more feasible, desirable design. While there exist many codes and regulations that will influence and inform the design of the
proposed structures, an important consideration is the University’s vision for the property. With proposed aspects of residential space included in the team’s building design, representatives from Residential Services will be interviewed to gain a better understanding of this department’s wants and needs for future residential structures. Additionally, the team plans to conduct interviews with representatives of the WPI Facilities Department to gain more information about the current site layout.

A solid baseline of information regarding Salisbury Estates will be gathered from these various meetings, a final consensus will be agreed upon and the scope of work outlined. Understanding that the entirety of Salisbury Estates will be demolished and leveled, a new proposal for the site layout of the property will be developed. Before coming to a final decision, different site plans and ideas will be considered. The final decision will incorporate shared points gathered from the interviews as well as engineering judgement used by the team.

After demolition, a parking lot will be added to account for the new residential and academic facilities while also forming a northward connection to Sagamore Road to provide an additional entrance/exit. In addition to the parking lot, both our designed residential and academic structures will need to be placed on the existing site. An iterative approach will be taken to design a site plan that will incorporate all aspects of the team’s design in an ideal manner.
3.2 Structural Design and Analysis

Structural design calculations will be conducted for both the academic and residential facilities. These calculations will be carried out starting with the top of the structure and then working downward. The roof will be designed to service design loads prescribed in ASCE 7-10 and the Massachusetts State Building Code, including snow, rain, earthquake, and dead loads. With the roof designed, the top floor members can be designed to carry this overhead loading along with its own applicable dead and live loads. This process will be applied to all additional floors before designing the foundation. LRFD design will be used throughout.

Structural analysis software, such as Risa 2D, will be used to analyze the integrated structural system selected through the previous design calculations. This will allow for the identification of errors and potential need for more design iterations until deflections and stresses are within allowable values.

3.3 Life Safety Analysis

Upon completion of the architectural drawings for both the residential and academic buildings, a comprehensive life safety analysis will be completed which will address evacuation plans, emergency lighting locations, exit sign placement, fire alarm pull stations, and occupant load calculations. Utilizing the requirements and specifications found from code research, AutoCAD drawings will be developed to show evacuations routes, and placement of emergency lighting, exit signs, and manual fire alarm pull stations. A sprinkler layout will be overlaid on the architectural drawings, for both the academic and residential buildings. Finally, occupant load placards will be developed for each room of both buildings based on occupancy classification and square footage.

3.4 Cost analysis

A comprehensive cost analysis shall be conducted for the demolition of the current buildings on the Salisbury Estates lot, the construction of the new buildings, and the projected return from the dining facility and the residential hall spaces.

3.4.1 Demolition

The demolition of the current apartments within Salisbury Estates, along with the leveling of the property, will contribute significantly to the cost of the project. Due to the focus of this project being on the structural design and life safety of the buildings, the specifics of the demolition process are not essential for the purpose of this project. Therefore, the cost estimate for this demolition will be calculated using an average cost per unit area of the property.
3.4.2 New Building

The construction of the new residential and academic buildings will comprise the majority of the cost for the project. The cost of the materials used will be researched and applied to the total amount of such materials used to determine this total material cost. Time to complete the construction will also be estimated to allow for a reasonable value for the cost of labor. Summing these values will provide an estimate for the cost of the project.

3.4.2.1 Materials

Once all of the structural members have been designed, the cost of the material used will be calculated using R.S. Means. Additionally, a list of interior materials used shall be provided upon completion of the building design. This list will contribute to the cost analysis portion of the report. Some examples of materials to be included are gypsum wall-board, carpeted flooring, and suspended ceiling tiles.

3.4.2.2 Labor Cost

The final component of the cost estimate of the building will be the labor cost of erection. It will be difficult to ascertain an accurate estimate of this cost due to unpredictable nature of the component costs. A time estimate will be made based on recently completed projects of a similar nature to this project. This time will be used to estimate the overhead cost of the construction with base values found from R.S. Means.

3.4.3 Return on Investment

With one of these buildings being residential with a small dining option, WPI will attain a source of income by investing in this property. The large increase in beds accompanied by the additional meal plans of the undergraduate students will greatly increase the revenue of this site. After a given amount of time, this development will provide WPI with profits which can then be used for further expansions.
4. Deliverables

One of the major deliverables provided will be the calculations carried out for the design of the structural members. The deliverables with respect to fire and life safety shall include AutoCAD drawings of the architectural floor plans to be overlaid with a sprinkler layout, stairwells, fire doors, and fire barrier locations. An occupancy load table for the buildings, a fire resistance material rating for the buildings, and site plan view displaying building entrances, pathways, fire access roads, as well as fire department connections will also be provided. AutoCAD and Revit models will also be produced to visually show the iterations and final designs of the proposed buildings. A cost estimate will supplement the design work to present the financial assessments for each necessary component of the project. A final report will be developed to demonstrate the actual methods used as well as corresponding results and conclusions made by the team. The entirety of this information will be organized visually in a clear and concise manner on a final poster. A schedule showing tasks to be completed throughout the three terms can be seen in Figure 2 below:

*Figure 2: Gantt Chart of Three-Term Schedule*
5. Bibliography


“FEMA Flood Map Service Center” FEMA, msc.fema.gov/

City of Worcester Zoning Ordinance 2018 Edition


Massachusetts State Building Code (MSBC) 9th Edition


National Fire Protection Association NFPA 72 2013 Edition

National Fire Protection Association NFPA 221 2015 Edition

National Fire Protection Association NFPA 80 2013 Edition

National Fire Protection Association NFPA 70 2014 Edition
Appendix B: Site Plan Brainstorm 1
Appendix C: Site Plan Brainstorm 2
Appendix D: Site Plan Brainstorm 3
Appendix E: Architectural Drawings

Appendix E.1: Residential Architectural Drawings
Second Floor

OL = 270 people
OL = 136 beds
Appendix E.2: Academic Architectural Drawings

First Floor

OL = 926 people

Lecture Hall
Classroom
Egress Stairs
Classroom
Classroom
Classroom
Classroom
Lecture Hall
Egress Stairs
Lecture Hall
Classroom
Classroom
Classroom
Egress Stairs
Classroom
Lecture Hall
Egress Stairs

Women’s Bathroom
Men’s Bathroom
Lounge Area
Lounge Area
Elevator
Elevator
Elevator
Elevator
Hvac
Hvac
Electric closet
Electric closet
Main Star
Main Star

Men’s Bathroom
Women’s Bathroom

Mach.
Mach.

Mach.
Mach.

Mach.
Mach.

Mach.
Mach.

Mach.
Mach.

Mach.
Mach.

Appendix F: Academic and Residential Sprinkler Design Drawings

Appendix F.1: Residential Sprinkler Design Drawings
Appendix F.2: Academic Sprinkler Design Drawings
Second Floor
## Appendix G: Academic Building Beam and Girder Design

### Appendix G.1: Beam Design Aid

<table>
<thead>
<tr>
<th>Initial Assumptions:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>$E_s$</td>
<td>29000 ksi</td>
</tr>
<tr>
<td>$F_y$</td>
<td>50 ksi</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Bay Properties:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>Section Length</td>
<td>10 ft</td>
</tr>
<tr>
<td>Section Spacing</td>
<td>10 ft</td>
</tr>
<tr>
<td>Is Interior? (1=No, 2=Yes)</td>
<td>1</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Selected Section Properties:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>Section</td>
<td>W2x4</td>
</tr>
<tr>
<td>Self Weight</td>
<td>10 lb/ft</td>
</tr>
<tr>
<td>Section $Z_y$ Value</td>
<td>10 in$^2$</td>
</tr>
<tr>
<td>Section $I_y$ Value</td>
<td>1140 in$^3$</td>
</tr>
<tr>
<td>Adjusted Load Combination ($W_u$)</td>
<td>1664.000 lb/ft</td>
</tr>
<tr>
<td>Adjusted Max Moment</td>
<td>249.600 k-in</td>
</tr>
<tr>
<td>New Acceptable $Z_x$</td>
<td>5.55 in$^3$</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Dead Loads:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>MEP</td>
<td>10 psf</td>
</tr>
<tr>
<td>Ceiling</td>
<td>10 psf</td>
</tr>
<tr>
<td>Concrete Slab+Decking</td>
<td>10 psf</td>
</tr>
<tr>
<td>Exterior Enclosure (DL)</td>
<td>10 lb/ft</td>
</tr>
</tbody>
</table>

| Total Dead Load | 310 lb/ft |

<table>
<thead>
<tr>
<th>Live Loads:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>Above Corridors</td>
<td>80 psf</td>
</tr>
<tr>
<td>Reduced Live Load</td>
<td>140.000 psf</td>
</tr>
</tbody>
</table>

| Total Live Load | 80.000 psf |
| Total Live Load | 800.000 lb/ft |

<table>
<thead>
<tr>
<th>Load Combinations:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>1.4(DL)</td>
<td>434 lb/ft</td>
</tr>
<tr>
<td>1.2(DL) + 1.6(LL) + 0.5(Lr or S or)</td>
<td>1652 lb/ft</td>
</tr>
</tbody>
</table>

| Used Combination ($W_u$) | 1652 lb/ft |
| Max Moment              | 247.8 k-in |

<table>
<thead>
<tr>
<th>Deflection Checks:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>$L/360$</td>
<td>0.333333333 in</td>
</tr>
<tr>
<td>$L/240$</td>
<td>0.5 in</td>
</tr>
<tr>
<td>$0.5LL$</td>
<td>400.000 lb/ft</td>
</tr>
<tr>
<td>$DL+0.5LL$</td>
<td>720.000 lb/ft</td>
</tr>
<tr>
<td>$\Delta_{max,LL}$</td>
<td>0.0027 in</td>
</tr>
<tr>
<td>$\Delta_{max,DL+LL}$</td>
<td>0.0049 in</td>
</tr>
</tbody>
</table>
### Appendix G.2: Girder Design Aid

<table>
<thead>
<tr>
<th>Initial Assumptions:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>Es</td>
<td>29000 ksi</td>
</tr>
<tr>
<td>Fy</td>
<td>50 ksi</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Beams within Bay Properties:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>Beam Length</td>
<td>35 ft</td>
</tr>
<tr>
<td>Beam Spacing</td>
<td>11.5 ft</td>
</tr>
<tr>
<td>Beam Self Weight</td>
<td>10 lb/ft</td>
</tr>
<tr>
<td>Is Interior? (1=No, 2=Yes)</td>
<td>1</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Girders within Bay Properties:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>Girders Length</td>
<td>46 ft</td>
</tr>
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</table>

<table>
<thead>
<tr>
<th>Selected Section Properties:</th>
<th>Units</th>
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</thead>
<tbody>
<tr>
<td>Section</td>
<td>W2x4</td>
</tr>
<tr>
<td>Self Weight</td>
<td>100 lb/ft</td>
</tr>
<tr>
<td>Section Z, Value</td>
<td>10 in</td>
</tr>
<tr>
<td>Section I, Value</td>
<td>1140 in²</td>
</tr>
<tr>
<td>Self Weight Distributed Load Factored (Wu)</td>
<td>120,000 lb/ft</td>
</tr>
<tr>
<td>Adjusted Max Moment</td>
<td>9589.59 lb-ft</td>
</tr>
<tr>
<td>New Acceptable Zx</td>
<td>213.10 in²</td>
</tr>
</tbody>
</table>

### Dead Loads: | Units |
<table>
<thead>
<tr>
<th></th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>MEP</td>
<td>10 psf</td>
</tr>
<tr>
<td>Ceiling</td>
<td>10 psf</td>
</tr>
<tr>
<td>Concrete Slab+Decking</td>
<td>10 psf</td>
</tr>
<tr>
<td>Exterior Enclosure (DL)</td>
<td>10 lb/ft</td>
</tr>
</tbody>
</table>

| Total Dead Load | 365 lb/ft |  

### Live Loads: | Units |
<table>
<thead>
<tr>
<th></th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>Above Corridors</td>
<td>80 psf</td>
</tr>
<tr>
<td>Reduced Live Load</td>
<td>79.813 psf</td>
</tr>
</tbody>
</table>

| Total Live Load | 79.813 lb/ft |

<table>
<thead>
<tr>
<th>Load Combinations:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>1.4(DL)</td>
<td>511 lb/ft</td>
</tr>
<tr>
<td>1.2(DL) + 1.6(LL) + 0.5(Lr or 5 or R)</td>
<td>1906.57 lb/ft</td>
</tr>
<tr>
<td>Used Combination (Wu) acting on Beam</td>
<td>1906.57 lb/ft</td>
</tr>
<tr>
<td>Concentrated load on Girder from Beam</td>
<td>58729.8 lb</td>
</tr>
<tr>
<td>Max Moment</td>
<td>9208.71 lb-ft</td>
</tr>
</tbody>
</table>
Appendix G.3: Academic Building Structural Bay Layout
Appendix G.4: Academic Building Final Member Sizes
Appendix G.5: Academic Building Roof Member Design

Interior Beam - Strength Performance

Load Design

- Roof: 70 psf (5)
- Wind: 31.5 psf (W)
- Snow: 55 psf (S)
- Live: 15 psf (L)

Steel Properties
- E = 30,000 ksi
- Fy = 50 ksi

Load Combination
1. wu = 1.40 = [1.4(3.15 psf + 16 psf + 8.85 psf)] 11.5 ft
   wu = 450.8 lb/ft
   wu = 450.8 lb/ft < 500 lb/ft, disregard
2. wu = 1.20 + 1.6 (L + O + S) + 1/2 H (L + O + S) of 0.5 w
   wu = [1.2(28 psf) + 1.6(55 psf)] 11.5 ft
   wu = 398.4 lb/ft governs

Calculate Mu

\[ Mu = \frac{wu \times b}{8} \]

Determine 2x using 2x \geq \frac{Mu}{Fy}

2x \geq \frac{398.4 \text{ lb}}{50 \text{ ksi}} \rightarrow 2x \geq 8.0 \text{ in}
Use Table 3-2 to find beam size

From Table 3-2: \[ w_2 = 0.94 \text{ in} \ (24 = 95.4 \text{ in}^3) \]

Update \( w_u \) with self-weight

\[ w_u = (139.8 \text{ lb/ft}) + 1.2(0.4 \text{ in} \ 16 \text{ ft}) = 1451.20 \text{ lb/ft} \]

Calculate new \( K_u \)

\[ K_u = \frac{w_u}{6} = \frac{(1451.20 \text{ lb/ft})(35 \text{ ft})}{6} = 7666.58 \text{ ft-in} \]

Check 2x

\[ 2x = \frac{K_u}{Q_y} = \frac{7666.58 \text{ ft-in}}{0.4 \text{ lb/ft} \ (50 \text{ kips})} = 39.3 \text{ in}^3 \]

\[ 2x \geq 39.3 \text{ in}^3 \]

\[ 2x = 95.4 \text{ in}^3 \]
Interior Beam - Deflection Performance

Live Loads

\[ w_l = \frac{1}{340} \text{ or } 1" \text{ max.} \]

\[ w_l = \frac{5uw}{384EI} \quad w = \text{roof live load (snow since grooms)} \]

\[ w_l = (25 \text{ kPa}) 11.5 \text{ ft} = 637.5 \text{ lb/ft} \]

\[ w_l = \frac{5(637.5 \text{ lb/ft})(18 \text{ in/ft})^2 (12 \text{ in/ft})^3}{384(2.0 \times 10^8 \text{ lb/ft}^2)(843 \text{ in}^3)} \]

\[ w_l = 0.87 \text{ in} \]

\[ L = \frac{35.4 + (18 \text{ in/ft})}{360} = 0.17 \text{ in} = \text{wcm} \text{ in} \]

\[ w_2 = 0.87 \text{ in} \times 2.0 = \frac{1.74}{360} \]

Dead & Live Loads

\[ w_1 = \frac{1}{240} \]

\[ w_1 = \frac{5uw}{234EI} \quad w_1 = \text{dead & live load} \]

\[ w_1 = 1.37g + 414 + 637.5 + 18g = 998.5 \text{ lb/ft} \]

\[ w_1 = \frac{5(998.5 \text{ lb/ft})(12 \text{ in/ft})^4 (12 \text{ in/ft})^3}{384(2.0 \times 10^8 \text{ lb/ft}^2)(843 \text{ in}^3)} \]

\[ w_1 = 1.38 \text{ in} \]

\[ \frac{1}{360} = \frac{25.4 + (18 \text{ in/ft})}{360} = 0.75 \text{ in} \]

\[ d_1 = 1.38 \times 2.0 = \frac{2.76}{360} \]

Use w = 1.25 x 94
Interior Girder - Strength Analysis

\[ Ru = \frac{wL^2}{2} \]

\[ Ru = \frac{145 (150 \times 12)}{(3.5 \times 12)} \Rightarrow Ru = 25.396 \text{ kN} \]

\[ F_u = 2 (35350 \text{ kN/m}) \Rightarrow F_u = 50.702 \text{ kN/m} \]

Calculate \( Ru \)

\[ Ru = \frac{wL^2}{12} + F_u = \left( \frac{150 \times 12}{12} \right) + \frac{50.702 (11.5 + 4)}{14} \Rightarrow 12 \text{ kN} \]

\[ Ru = 14018.59 \text{ kN/m} \]

Determine \( 2 \times \) using \( 2 \geq \frac{Ru}{F_f} \)

\[ 2 \geq \frac{14018.59}{140} \Rightarrow 2 \approx 101.3 \text{ in}^3 \]

Use Table 3 to find girder size

From Table 2.1 \( \Rightarrow 0.33 \times 130 \) \( (2w = 467 \text{ in}^3) \)

\[ \omega = \frac{0.33 \times 130}{60} \Rightarrow \omega = 156 \text{ lb/ft} \]

\[ H_{ncf} = \frac{wL^2}{8} = \left( \frac{156 \text{ lb/ft}}{60 \text{ lb/ft}} \right) \left( 11.5 \text{ ft} \right)^2 \Rightarrow Hu = 951.144 \text{ kN-m} \]

\[ Ru = 2 (14018.59 \text{ kN/m}) \Rightarrow Ru = 14513.736 \text{ kN-m} \]

Check \( 2 \times \)

\[ 2 \geq \frac{14513.736}{140} \Rightarrow 2 \geq 103.7 \text{ in}^3 \]

Check \( 2 \times \)

\[ 2 \geq \frac{14513.736}{140} \Rightarrow 2 \geq 103.7 \text{ in}^3 \]

2 x required: \( 323.3 \text{ in}^3 \geq 167.0 \text{ in}^3 \Rightarrow \omega = 33 \times 130 \)
Interior Girder - Deflection Performance

Live Loads (LL), no 0.5 single 100 ft

\[ DL \leq \frac{L}{180} \text{ or } 1^\prime \text{ max.} \]

\[ DL = \left( \frac{PL^3}{4EI} + \frac{PL}{EI} \left( 3L^2 - 4a^2 \right) \right) \]

\[ DL = \left( \frac{2440 \times 10^3 \times (4.5)^3}{4 \times 10^3 \times 10^4 \times (30000 \times 10^{-6})} + \frac{2440 \times 10^3}{10^4 \times 10^4 \times (30000 \times 10^{-6})} \left( 3(4.5)^2 - 4 (1.5 \times 4.5)^2 \right) \right) \frac{1738\text{ in}^3}{144^3} \]

\[ DL = 0.95 \text{ in} \]

\[ \frac{L}{360} = \frac{46 \text{ ft} \times (12 \times 14)}{360} = 1.17 \text{ in} = \text{ use } 1 \text{ in.} \]

\[ DL = 0.95 \text{ in} \times 1 \text{ in} = \frac{1}{360} \]

Dead & Live Loads (DL+LL), no 0.5 single 100 ft

\[ DT \leq \frac{L}{240} \]

\[ DT = \left( \frac{PL^3}{4EI} + \frac{PL}{EI} \left( 3L^2 - 4a^2 \right) \right) \]

\[ DT = \left( \frac{(46 \times 10^4) \times (4.5)^3}{4 \times 10^3 \times 10^4 \times (30000 \times 10^{-6})} + \frac{(46 \times 10^4)}{10^4 \times 10^4 \times (30000 \times 10^{-6})} \left( 3(4.5)^2 - 4 (1.5 \times 4.5)^2 \right) \right) \frac{1738\text{ in}^3}{144^3} \]

\[ DT = 1.92 \text{ in} \]

\[ \frac{L}{360} = \frac{46 \text{ ft} \times (12 \times 14)}{360} = 3.83 \text{ in} \]

\[ DT = 1.92 \text{ in} \times 3.83 \text{ in} = \frac{1}{240} \]

\[ \text{Use W 38 \times 130} \]
Exterior Beam / Girder
Followed some processes
Exterior enclosure WA

Beam = W31 x 44
Girder = W33 x 130
Academic Building  Interior Edge Bay  Exterior Beam  \( \frac{1}{2} \)

\( E_s = 29,000 \, \text{ksi} \), \( S = 500 \, \text{ksi} \)

Beam length: 30 ft, Span: 11.5 ft

- DL:
  28.50 ft
  20.00 ft
  5.50 ft

Tributary Area: \( 11.5 \times 5 = 57.5 \, \text{ft}^2 \)

Loads:

- DL = (28) (5.75) = 161.25 lb/ft
- S = (55) (5.75) = 316.25 lb/ft

Load combination:

- 1.2DL + 1.6S = 1.2(161.25) + 1.6(316.25) = 193.2 + 506 = 699.2 lb/ft
- \( w_o = 699.2 \, \text{lb/ft} \)
- \( M_o = \frac{1627.2 \times 30^2}{8} \left( \frac{1}{1000} \right) \left( \frac{12}{1} \right) = 943.92 \, \text{lb-ft} \)

- \( z_x = \frac{943.92}{(50)} = 20.88 \, \text{in}^3 \)

- \( I_x = 130 \, \text{in}^4 \)

- DL = 161.25 \times 11 = 1805 \, \text{lb/ft} \)
- 1.2DL + 1.6S = 1.2(1805) + 1.6(316.25) = 2162.5 + 506 = 722 \, \text{lb/ft} \)

- \( M_o = \frac{172.2 \times 30^2}{8} \left( \frac{1}{1000} \right) \left( \frac{12}{1} \right) = 1947.75 \, \text{lb-ft} \)

- \( z_y = \frac{1947.75}{(50)} = 40.35 \, \text{in}^3 \)

- 24.7 in \geq 21.6 \, \text{in}
\[ \Delta T = \frac{5.2 + 1.5}{20} = 1.5 \text{ in} \]

\[ u_i = (20 + 65) (5.75) + 19 = 496.25 \text{ W/ft}^2 \]

\[ \Delta T = \frac{5 \frac{4}{12} (30)^4}{38.4 \times 10^{-8} (28000)(180)} \left( \frac{1}{1000} \right) \left( \frac{12}{1} \right)^3 = 1.291 \geq 1.5 \]

*I required* = 1.30 \left( \frac{2.4}{1.5} \right) = 207.91 \text{ in}^4 \Rightarrow \text{W114 x 26} \quad I_s = 245 \text{ in}^4 \]

\[ W_1 = (26155)(5.75) + 26 = 503.25 \text{ lb/ft} \]

\[ \Delta T = \frac{5 (53.25)(30)^4}{38.4 \times 10^{-8} (28000)(180)} \left( \frac{1}{1000} \right) \left( \frac{12}{1} \right) = 1.291 \geq 1.5 \]

\[ \Delta s = \frac{1}{2} \frac{1}{4} \text{ or } 1' = 1' \]

\[ u_s = 55 (5.75) = 316.25 \]

\[ L = \frac{5 (316.25)(30)^4}{38.4 \times 10^{-8} (28000)(180)} \left( \frac{1}{1000} \right) \left( \frac{12}{1} \right)^3 = 0.811.7 \geq 1' \]

\[ W_1 = \text{W94 x 26} \]
Academic Building | Interior Edge Bay | Interior Beam 1/2

\[ E_s = 29,000 \text{ ksi}, \quad E_I = 50 \text{ ksi} \]

Beam length = 30 ft, Span = 41.5 ft

DL = 1.4DL + 1.6(1.0 or S) = 1.4(322) + 1.6(230)

Load Combinations

1. DL = 1.4(322) = 450.8 kips
2. 1.2DL + 1.6(1.0 or S) = 1.2(322) + 1.6(230) = 386.4 + 368 = 754 kips

\[ w_o = \frac{386.4 \times 41}{60} = 1645.58 \text{ kips/ft} \]

\[ M_o = \frac{w_o L^2}{2} = \frac{1645.58 \times (41)^2}{2} = 155,295 \text{ kips-ft} \]

\[ M_{u,q} = 1645.58 \times 12 \text{ in} = 18,635 \text{ kips-ft} \]

\[ Z_{x} \geq \frac{M_{u,q}}{\phi_E} = \frac{18,635 \times 5.0}{(0.9)(5.0) \text{ kips}} = 41.41 \text{ in}^3 \]
Checks AISC Manual Table 3-2 for min Zx value greater than 41.97 in^3 (Bold W shape has lowest weight)

Use W14×80 \[ Z_x = 47.3 \text{ in}^3 \quad I_x = 29.1 \text{ in}^4 \]

Add 30 16/4 \[ \text{DL} = 322.5 \cdot 30 = 35216.8 \text{ in}^3 \]

1.2 DL + 1.6 S = 1.2(352) + 1.6(632.5) = 422.4 + 1012 = 1434.4 \text{ in}^3

\[ W_0 = 1434.4 - 16/4 \]

\[ M_0 = \frac{1434.4}{6} \cdot \left( \frac{12}{1000} \right) = 1986.49 \text{ in} \cdot \text{in} \]

\[ Z_x = \frac{1986.49}{0.9/30} = 47.3 \text{ in}^3 \quad \text{47.3} > 43.0 \text{ in}^3 \]

Deflection Checks

\[ \Delta T = \frac{5wL^4}{384EI} \]

\[ \Delta T = 5 \cdot \frac{W_0}{5} \cdot \frac{L}{2} = 5 \cdot \frac{(1434.4)(12)}{384 \cdot 29000 \cdot (0.9)} = 1.23 \quad \frac{\text{in}}{\text{kip in}^4} \]

\[ \text{Kin} = \frac{60}{1000} \cdot \frac{1}{1000} \left( \frac{12}{12} \right) = 0.000 \]

\[ \Delta T = 2.126 \text{ in} \quad \frac{2.126}{0.000} = \frac{2.126}{0.000} \left( \frac{12}{12} \right) = 15 \text{ in} \quad 2.126 \times 15 \]

Required \[ I_x = 29.1 \cdot \left( \frac{32}{4} \right) = 412.17 \text{ in}^4 \quad \text{W18×35} \quad I_x = 510 \text{ in}^4 \]

\[ W_0 = 3545.36 - 30 = 3515 \text{ kip} \]

\[ \Delta T = 5 \cdot \frac{9845.50}{384 \cdot (29000)(0.9)} \cdot \left( \frac{12}{12} \right)^3 = 1.22 \text{ in} \leq 1.5 \text{ in} \]

\[ \Delta S \leq \frac{w_0}{250} = 1'' \]

\[ w_3 = \frac{55.0}{12} = 632.5 \]

\[ \Delta S = \frac{5(632.5)(0.9)}{384 \cdot (29000)(0.9)} \cdot \left( \frac{12}{12} \right)^3 = 0.76 \quad \frac{250}{w_0} = 30 \quad 250 = 0.76 \leq 1 \]

Use W18×35
Appendix G.6: Academic Building Third/Second Floor Member Design

Interior Beam - Service Performance

Loads

- Deck: 7 psf
- Slab: 35 psf
- Corridors: 10 psf

Reduced live load: \( L = 1.0 \left( \frac{0.35 + 1.5}{V_{k+0.25}} \right) \)

Load combinations:

1. \( w_u = 1.40 = (1.4(35 psf + 3.6 psf) + 10 psf) = 58.8 \text{ psf} \)
2. \( w_u = 1.20 \cdot 1.4 \cdot 0.5 \text{ (corridors) = 8 psf} \)

Calculate \( w_n \):

\[ w_n = \frac{w_u}{k} = \frac{1559.32 \text{ psf}}{0.85} = 1820.85 \text{ k-in} \]

Determine \( 2x \) using \( 2x \geq \frac{w_n}{F_y} \):

\[ 2x \geq \frac{1820.85 \text{ k-in}}{50 \text{ k-in}} = 36.4 \text{ k-in} \]
Use Table 3.2 to find beam size.

From Table 3.2: $w = 18 \times 40 \ L_2 = 78.4 \ m^3$

Update $w_n = 1 \ self-weight$

$w_n = (1550 \times 1 + 1600 \times 1) \times 1.2 + (40 \times 0.9 + 0.6) \times 1.2 \Rightarrow w_n = 17007.2 \ N/m^2$

Calculate new $u_n$

$u_n = \frac{w_n b^2}{d} = \frac{(17007.2 \times 1.141 \times 0.65)}{25.4^2} \Rightarrow u_n = 9953.45 \ kN/m$

Check $2a$

$2a \geq \frac{u_n}{f_y} = \frac{(9953.45 \ kN/m)}{50 \ kN/m^2}$

$2a \geq 0.161 \ m^3$ \hspace{1cm} $2a = 0.18 \ m^3$ ✗
**Interior Beam - Deflection Performance**

**Live Loads**

\[ a_l \leq \frac{1}{8} \text{ or } 1'' \text{ max} \]

\[ a_l = \frac{5wEL^4}{384EI} \quad w_l = \text{corridors load since governs} \]

\[ w_l = 0.5(0.744\text{ lb/ft}) = 0.5 \times 312.35 = 156.175 \text{ lb/ft} \]

\[ a_l = \left( \frac{5(312.35)(1.06)(3.011)^4}{384(39.10^6)(0.12^2)} \right) \frac{1736.1^3}{144} \approx 0.59 \text{ in} \]

\[ \frac{L}{300} = \frac{2544 \times (12.5)(144)}{360} = 1.17 \text{ in} \quad \text{not 2 in} \]

\[ a_l = 0.59 \text{ in} \leq 1 \text{ in} = \frac{1}{300} \]

**Dead 2 Live Loads**

\[ a_t \leq \frac{1}{240} \]

\[ a_t = \frac{5w_0L^4}{384EI} \quad w_0 = \text{dead 2 live load} \]

\[ w_0 = (466.5 \text{ lb/ft} + 40(12/144)) + (312.35(11/144)) \Rightarrow w_0 = 818.85 \text{ lb/ft} \]

\[ a_t = \left( \frac{5(818.85)(1.06)(3.011)^4}{384(39.10^6)(0.12^2)} \right) \frac{1736.1^3}{144} \approx 1.56 \text{ in} \]

\[ \frac{L}{360} = \frac{354(12.5)(144)}{360} = 1.75 \text{ in} \]

\[ a_t = 1.56 \text{ in} \leq 1.75 \text{ in} = \frac{1}{360} \]

[Use W18 x 40]
Interior Girder - Strength Performance

\[ A_u = \frac{wL}{f} \]

\[ A_u = \frac{(160.73 \text{ kips})(3524)}{28.138} \Rightarrow A_u = 28.138 \text{k} \]

\[ A_u = 2 \times (28.138 \text{k}) \Rightarrow 2A_u = 56.256 \text{k} \]

Calculate \( M_u \)

\[ M_u = Pa = \left( \frac{56.256 \text{k}}{0.33 \text{ ft}} \right) \text{ft} \text{k} \]

\[ M_u = 6298.433 \text{ k-f} \text{t} \text{m} \]

Determine \( Z \) using \( Z = \frac{\mu L}{\psi} \)

\[ Z = \frac{(6298.433 \text{ k-f})}{(0.9)(50 \text{ ksf})} \Rightarrow Z = 140.0 \text{ m}^3 \]

Use Table 3-3 to find girder size

From Table 3-3 \( \Rightarrow W = 65 \times 6.2 \) (2x = 199 m^3)

\[ w_{self} = 1.2(65 \times 6.2) \Rightarrow w_{self} = 74.4 \text{ lb/ft} \]

\[ M_{self} = w_{self} L^2 = \left( \frac{74.4 \text{ lb/ft}}{8} \right)(2.5)^2 \Rightarrow M_{self} = 87.944 \text{ k-f} \text{t} \text{m} \]

\[ M_u = (6298.433 \text{ k-f} \text{t}) + (87.944 \text{ k-f} \text{t} \text{m}) \Rightarrow M_u = 6385.977 \text{ k-f} \text{t} \text{m} \]

Check \( 2x \)

\[ 2x = \frac{(6385.977 \text{ k-f} \text{t} \text{m})}{(0.9)(50 \text{ ksf})} \Rightarrow 2x = 141.3 \text{ m}^3 \]

\[ 2x \text{ required} = 141.3 \text{ m}^3 < 149 \text{ m}^3 \Rightarrow x = 6.7 \text{ m}^3 \]
Interior Girders - Deflection Resistance

**Live Loads** (0.5LL)

\[ DL = \frac{1}{360} \quad \text{in} \]

\[ DL = \left( \frac{p_0}{D(EI)} \right) (3L^2 - 4a^2) \]

\[ DL = \left( \frac{(10.93\times10^4)(9.33\times10^4)}{3\times(30.16 \times 10^6)(1830 \times 10^9)} \right) \frac{1728 \times 10^3}{1 \times 12} \]

\[ DL = 0.33 \text{ in} \]

\[ L = \frac{26.2 + (10m)(14)}{360} = 0.93 \text{ in} \]

\[ DL = 0.33 \times L = 0.30 \times \frac{1}{360} \]

**Dead & Live Loads** (DL + 0.5LL)

\[ DL = \frac{1}{360} \]

\[ DL = \left( \frac{p_0}{D(EI)} \right) (3L^2 - 4a^2) \]

\[ DL = \left( \frac{(216.85 \times 10^4)(9.33 \times 10^4)}{3\times(30.16 \times 10^6)(1830 \times 10^9)} \right) \frac{1728 \times 10^3}{1 \times 12} \]

\[ DL = 0.94 \text{ in} \]

\[ L = \frac{26.2 + 10m(14)}{360} = 1.40 \text{ in} \]

\[ DL = 0.94 \times L = 0.94 \times \frac{1}{360} \]

\[ \text{Use } w = 41 \times 60 \]
Exterior Girder

Followed same process.

Added exterior enclosure weight:

38 psf (brick veneer) \times 15 \text{ ft (story height)} = 570 \text{ lb ft}

Girder \Rightarrow W_{24 \times 84}
Appendix G.7: Member Design Around Main Staircase

Academic Building | Main Stair Beam System

\[ E = 29,000 \text{ ksi} \quad f_y = 50 \text{ ksi} \]

**DL**

Load

\[ DL = (8.85 + 1.2 \times 40) (30) = 1555.5 \text{ kips} \]

**LL**

Load

\[ LL = \frac{1000}{30} (30) = 3000 \text{ kips} \]

Do not use Live Load Reduction for stairways.

**Load Combinations**

1.4 \( DL = 1.4 \times 1555.5 = 2177.7 \text{ kips} \)

1.2 \( DL + 1.6 \times LL = 1.2 \times 1555.5 + 1.6 \times 3000 = 8066.6 \text{ kips} \)

\[ M_0 = \frac{d \times 22^2}{8} \times \left( \frac{12 \times 1.1}{10000} \right) = 4839.95 \text{ kip-ft} \]

\[ M_0 = 839.95 \text{ kip-ft} \]

\[ Z_x = \frac{M_0}{f_y} = \frac{4839.95}{50} = 96.79 \text{ in}^3 \]
Check AISC Table 3-2 for min 2x value greater than 102.55 in²

Use W21×50, 3x = 110 in³, Jx = 984 in⁴

Add 50 lb/ft to DL

DL = (1.555,5 + 57z) = 1605.5 lb/ft

1.2DL + 1.6LL = 1.2(1605.5) + 1.6(3,000) = 1926.6 + 4800 = 6726.6 lb/ft

M0 = \( \frac{6726.6 \times 22}{3} \times \left( \frac{1}{1000} \right) \times \left( \frac{12}{1} \right) = 4883.5 \) in·ft

2x = \( \frac{4883.5}{110} = 108.5 \) in³

Design Check:

\( \Delta_T = \frac{22}{240} = 0.1 \) in

\( w_0 = \frac{D + 0.05L}{(2055) + 0.5(3000)} \)

\( w_T = \frac{(1605.5) + (1500)}{3110.5} = 0.57 \) in ≤ 0.1 in

\( L_e = \frac{22 \times (12)}{360} = 0.733 \) in

\( L = 0.5L = 1500 \) in

\( L_e = \frac{5(1500)}{39} \times \left( \frac{1}{29000} \right) \times \left( \frac{12}{1000} \right) = 0.777 \) in ≤ 0.753 in

Use W21×50 member for the horizontal beam
Academic Building  |  Main Steel Beam System  |  Workers

\[ \begin{align*}
E_s &= 29,000 \text{ kips} \\
K_t &= 2 \text{ (Interior Beam)}
\end{align*} \]

Beam Length = 3 ft
Tebular width = \( \frac{936}{2} = 4.685 \) ft
1 point load at midpoint

\[ \begin{align*}
W_0 &= 6726.6 \text{ lb} \\
D_{min} &= 6726.6 \left( \frac{2}{2} \right) = 73,992.6 \text{ lb} \\
R &= \frac{D_{min}}{2} = \frac{73,992.6}{2} = 36,996.3 \text{ lb} \\
M_w &= \frac{R}{y} = \frac{73,992.6}{4} \left( \frac{1}{1000} \right) \left( \frac{13}{12} \right) = 6,659.33 \text{ ft-lb}
\end{align*} \]

Distributed Load

DL = \( (8.85 + 12 + 40) \times \frac{465}{12} \) = 241.88 lb/ft

\[ \begin{align*}
\mu &= 100 \times 465 = 46,500 \text{ lb} \\
\mu_2 &= 80 \times 465 = 37,200 \text{ lb}
\end{align*} \]

Load Combinations

1.2 DL + 1.6 LL = 1.2 (241.88) + 1.6 (465) = 290.26 + 746.4 = 1036.66 kips

1.2 DL + 1.6 LL = 1.2 (241.88) + 1.6 (373.2) = 290.26 + 597.12 = 887.38 kips
Design Elevator cars as 9' x 6' 

Area: 
9' x 6' = 54 sq ft.

Using ASME A17.1-2007/CSA B44-07 Figure 8.2.1.2 
Minimum Rated Load For Passenger Elevators 
54.89^2 \rightarrow (5,500 \ lbs) \ \ \ LL = 1.6(5500)=8800 

Placed loads at location causing Max Moment and Deflection

1st Floor 

2nd Floor 

First Floor Moment 
\[ M_{max} = P_{e1} + P_{e2} + R \left( \frac{R}{2} \right) \left( 8.50h \right) \left( 6 \right) + \left( \frac{8.50h}{2} \right) \left( 6 + h \right) + \left( \frac{10.36h \left( 1/3 \right) \left( 6 + h \right)}{2} \right) + \left( \frac{20.88h \left( 1/2 \right) \left( 6 + h \right)}{3} \right) \]

\[ M_{max} = 5490 + 105600 + 3111.98 \left( \frac{12}{12+1.5} \right) = 159400 \rightarrow 11294.73 \]

\[ M_{max} = 200389.73 \left( 11.5 \right) \left( \frac{1/3}{12 + 1.5} \right) = 29104.62 \text{ kN-m} \]

Second Floor Moment 
\[ M_{max} = \frac{1584400}{2} - \left( \frac{88738.1}{6} \right) \left( 6 + h \right) - \left( \frac{R}{2} \right) \]

\[ M_{max} = 1584400 + 35938.89 = 194338.89 \left( \frac{1}{1000} \right) \left( \frac{12}{12+1.5} \right) = 2332575 \text{ kN-m} \]
100
\[ \Delta_{\text{max}} = \frac{5(84)(30)}{84(200)(200)} + (476.13)(6) \left( \frac{(30^2 - 4)(5^2 + 6^2)}{84(200)(200)} \right) + \]
\[ + \frac{(2.756)(6)}{6(1000)(2000)} \left( 3(30 \times 15) - 3(15)^2 - (6)^2 \right) + \]
\[ + \frac{(2.756)(12)}{6(1000)(2000)} \left( 5(30 \times 15) - 3(15)^2 - (12)^2 \right) + \frac{(5816.5)(30^3)}{43(2900)(2000)} \frac{1728 \text{ in}^3}{1 \text{ in}^3} \cdot \left( \frac{1}{1000} \right) + \frac{1}{15} \]

\[ \Delta_{\text{max}} = \left( 0.01289 + 0.02287 + 0.02657 + 0.04279 \times 0.2796 \right) \left( 1728 \right) \left( \frac{1}{1000} \right) \]

\[ \Delta_{\text{max}} = \left( 0.3834 \right) \left( \frac{1728}{1000} \right) = 0.6626 \text{ in} > 0.216 \]

\[ J_x \text{ needed} = 2370 \left( \frac{0.6626}{0.216} \right) = 7266.66 \text{ in}^4 \Rightarrow \frac{1}{3} \text{ in}^3 \times 135 \Rightarrow 7800 \text{ in}^4 \]

\[ \Delta_{\text{max}} = \left( 5 \left( \frac{135}{30} \right) \right) \left( \frac{3110}{7800} \right) + \frac{0.02289 \left( \frac{2370}{7800} \right)}{0.02567 \left( \frac{2370}{7800} \right)} + \]

\[ + \frac{0.04279 \left( \frac{2370}{7800} \right) + 0.2796 \left( \frac{2370}{7800} \right) \left( \frac{1728}{1000} \right)}{1000} \]

\[ \Delta_{\text{max}} = \left( 6.29 \times 10^{-4} + 0.00696 + 0.00777 + 0.01291 + 0.0849 \right) \left( 1.728 \right) \]

\[ \Delta_{\text{max}} = 0.1957 \text{ in} \leq 0.216 \text{ in} \]

Use: \[ W36 \times 135 \]
\[ w_T = 84 \text{ kips} \]

\[ \Delta_{\text{max}} = \left( \frac{5}{38.1 (24000)(2570)} \right) (428.48) \left( \frac{3}{15} \right) - \left( \frac{1}{15} \right) \]

\[ = \left( \frac{2750}{6 (24000)(2570)} \right) \left( \frac{3}{15} \right) \left( \frac{3}{15} \right) - \left( \frac{1}{15} \right) \]

\[ \Delta_{\text{max}} = \left( \frac{0.0129 \text{ in}^6}{0.0126 \text{ in}^6} \right) \left( \frac{1 \text{ kip}}{1000 \text{ kip}} \right) \]

\[ \Delta_{\text{max}} = 0.6587 \text{ in} > 0.2161 \text{ in}^5 \]

\[ I_x \text{ needed} = 2370 \left( \frac{0.6587}{0.0126} \right) = 1722.06 \text{ in}^4 \]

\[ W \geq 135 \]

\[ \Delta_{\text{min}} = 0.0027 \text{ in}^3 + 0.00627 + 0.00277 + 0.01921 + 0.08492 \]

\[ \Delta_{\text{max}} = 0.1257 \text{ in} \leq 0.2161 \text{ in} \]
Academic Building | Main Steel Beam System | Eclators

$E_s = 29,000 \text{ ksi}$, $E_f = 350 \text{ ksi}$

$K_u = 2$ (Given Beam)

Beam length = $50'$

Floor width (left side) $= \frac{9.86}{2} = 4.93'$

Right side same as left side from previous beam

---

Last side

Distributed Loads

\[ DL \]

- MEP = $8.85 \text{ psf}$
- Inclination = $1 \text{ psf}$
- Ceiling = $2 \text{ psf}$
- Decking + $546 = 90 \text{ psf}$

\[ DL = (8.85 + 1 + 2 + 90) (4.665) = 241.86 \text{ kips/ft} \]

\[ LL_{1st} = 100 (4.665) = 466.5 \text{ kips/ft} \]

\[ LL_{2nd} = 80 (4.665) = 373.2 \text{ kips/ft} \]

Load Combinations

\[ 1.2 DL + 1.6 LL_{1st} = 1036.66 \text{ kips/ft} \]

\[ 1.2 DL + 1.6 LL_{2nd} = 687.38 \text{ kips/ft} \]
First Floor Moment

\[ M_{1F} = \frac{w_1L^2}{8} = \frac{(1036.66)(30^3)}{8} \left( \frac{12}{1000} \right) = 1399.99 \text{ k-in} \]

Second Floor Moment

\[ M_{2F} = \frac{(887.38)(30^3)}{8} \left( \frac{12}{1000} \right) = 1197.963 \text{ k-in} \]

Total Moment

\[
\begin{align*}
1^{st} \text{ Floor} & \\
M &= 1399.99 + 2404.62 \\
&= 3804.61 \text{ k-in} \\
Z_z &= \frac{3804.61}{0.9(50)} = 84.54 \text{ k-in} \\

2^{nd} \text{ Floor} & \\
M &= 1197.963 + 2332.07 \\
&= 3530.03 \text{ k-in} \\
Z_z &= \frac{3530.03}{0.9(50)} = 78.45 \text{ k-in}^3
\end{align*}
\]

Use \( W \) = 1.21 x 44 + 2.9 = 94.94 k-ft

\[
\begin{align*}
\Delta &= 1.2 (44) = 52.8 \text{ k-ft} \\
W_0 &= 1036.66 + 92.8 = 1029.46 \text{ k-ft} \\
M_{1F} &= 1470.77 \text{ k-in} \\
M &= 1470.77 + 2404.62 = 3875.39 \text{ k-ft} \\
Z_z &= \frac{3875.39}{0.9(50)} = 86.12 \text{ k-in}^3
\end{align*}
\]

\[
\begin{align*}
\Delta_{1F} &= 1/1666 = 0.2161 \text{ in} \\
1^{st} \text{ Floor} & \\
W_0 &= 241.88 + 44 = 285.88 (166.5) = 519.13 \text{ k-ft} \\
2^{nd} \text{ Floor} & \\
W_0 &= 241.88 + 44 + 0.5(373.2) = 472.98 \text{ k-ft}
\end{align*}
\]
Academic Building | Main Stalk Beam System | Wagner

1st Floor

\[ \Delta_{max} = \left( 5 \left( \frac{19.13}{384 \text{ (2900)} \text{ (893)} \right) + \left( \frac{0.02964 + 0.03311 + 0.06505 \pi}{843} \right) \right) \left( \frac{1}{1000} \right) \left( \frac{12}{1} \right) \]

\[ \Delta_{max} = \left( 0.224 + (0.1178) (2.171) \right) \left( 1.728 \right) = (0.224 + 0.2557) (1.728) \]

\[ \Delta_{max} = 0.4477 \times 1.728 = 0.829 \text{ in} \geq 0.2161 \text{ in} \]

J_t needed = \[843 \times \left( \frac{0.829}{0.2161} \right) = 3.233.75 \text{ in}^4 \Rightarrow W \text{ 30 x 99} \]

I_t = 3990 \text{ in}^4

\[ w_t = 579.13 \times 44.99 = 579.13 \text{ in}^4 \]

\[ \Delta_{max} = \left( 5 \left( \frac{574.13}{384 \text{ (29000)} \text{ (893)} \right) + \left( \frac{0.2557 \times 843}{3990} \right) \right) \left( 1.728 \right) \]

\[ \Delta_{max} = \left( 0.0523 + 0.054 \right) \left( 1.728 \right) = (0.06632) (1.728) \]

\[ A_{max} = 0.18373 \leq 0.2161 \text{ in} \]

Use W \text{ 30 x 99}

2nd Floor

\[ \Delta_{max} = \left( 5 \left( \frac{72.98}{384 \text{ (29000)} \text{ (893)} \right) + \left( 0.02673 + 0.03311 + 0.06505 \right) \right) \left( \frac{12}{1} \right) \]

\[ \Delta_{max} = \left( 0.20389 + (0.1178) (2.171) \right) \left( 1.728 \right) = (0.20389 + 0.24936) (1.728) \]

\[ \Delta_{max} = (0.1532) \left( 1.728 \right) = 0.7831 \text{ in} \geq 0.2161 \text{ in} \]

J_t needed = \[843 \times \left( \frac{0.7831}{0.2161} \right) = 3.054.98 \text{ in}^4 \Rightarrow W \text{ 30 x 99} \]

105
Appendix G.8: Member Design Around Egress Staircase

Academic Building | Egress Stair System

\[ E = 29,000 \quad F = 50 \text{ km} \]
\[ k_{ne} = 2 \text{ (Aisle Bow)} \]
Beam Length = 23 ft
Top and Bottom = 8, 2, 8 = 15 ft

DL

<table>
<thead>
<tr>
<th>Load</th>
<th>Value</th>
</tr>
</thead>
<tbody>
<tr>
<td>Mp</td>
<td>8.85 psf</td>
</tr>
<tr>
<td>Insulation</td>
<td>2 psf</td>
</tr>
<tr>
<td>Ceiling</td>
<td>2 psf</td>
</tr>
<tr>
<td>Decking + slab</td>
<td>90 psf</td>
</tr>
</tbody>
</table>

\[ L_{r, 	ext{total}} = 1.6 \left(0.05 \times 15\right) \]

K = 2 for interior beams
\[ A_{11} = 2.3 \times 1.5 \]

Load Combination

\[ 1.4 \times DL = 1.4 \times 777.75 \times 15/8 = 1697.85 \text{ lb/ft}^{2} \]
\[ 1.2 \times DL + 1.6 (L_{r, \text{total}}) = 1.6 \times (1500) = 2400 = 3.333 \times 1.5 \]
\[ M_{0} = 3.333 \times 1.5 \]

Max Moment

\[ M_{0} = \frac{F \times L}{2} = \frac{(3.333) \times 12}{8} \times \left(\frac{1}{1000}\right) \times \left(\frac{12}{18}\right) = 2.644,97 \times 10^{-3} \]

\[ z = \frac{F}{2} \times \left(\frac{L}{12}\right) = 58.27 \times 10^{-3} \]
Academic Building | Eggss Stair System

Check AISI Manual Table 3.2 for min 2a value greater than
58.37 in

Use $\frac{u}{18x3.5}$, $z_a = 66.5$ in

Add $31 \frac{1}{2} + DL$

$DL = \frac{772.75 + 35}{3} = 812.75$ kips

$M_u = 33.75 \times \left(\frac{1}{100} \right) \left( \frac{12}{4} \right)^{\frac{3}{2}} = 2678.3$ kip-ft

$z_a = 2 \times \frac{2678.3}{10} = 59.52$ in

Deformation Checks

$\Delta L = \frac{1100}{240} = 1.15$ in

$w_1 = 0.5 L = 38.125 \times 1000 = 38125$ lbs

$W_3 = 0.5 (1500 \times 1000) = 750 \times 1000$ lbs

Use $\frac{u}{18x3.5}$ member for these horizontal beams
Academic Building | Stair System

\[ F_s = 21,000 \text{ kN}, \quad F_1 = 800 \text{ kN} \]
\[ k = 2 \text{ (unlighted beam)} \]

Beam length, \( L = 30' \)
Newel length, \( L = 11.5' \)

2 symmetric point loads

**Concentrated Loads**
\[ W_n = 3,833.3 \text{ kN} \]
\[ M_n = P_n \frac{L}{2} \times 12 = 58,855.5 \text{ kN-m} \]

\[ M_n = 38,333.3 \times (8.25) \times \frac{1}{12} = 3,745.8 \text{ kN-m} \]

**Distributed Load**

\[ DL = 0.85 \text{ psf} \]
\[ LL = 100 \text{ psf} \]
\[ LL_{0.25} = 60 \text{ psf} \]
\[ LL_{0.25} = 30 \text{ psf} \]

\[ \text{Load} \]
\[ DL = (0.85 \times 12 + 40) \times 5.75 = 298.14 \text{ kN/m} \]
\[ LL = (100) \times 5.75 = 575 \text{ kN/m} \]

**Load Combinations**

\[ 1.2 DL + 1.6 LL_{0.25} = 1.2(298.14) + 1.6(575) = 1357.77 \text{ kN/m} \]
\[ 1.2 DL + 1.6 LL_{0.25} = 1.2(298.14) + 1.6(460) = 1357.77 + 736 = 1093.77 \text{ kN/m} \]

** Additional Analysis**

\[ \text{KLC} = 2 \text{ for interior beam} \]

\[ \frac{F_n}{L} \text{ reaction} = \frac{LL_{0.25} + 15}{L} \]
\[ \Delta T \leq \frac{20}{240} = 0.08 \text{ in.} \]

\[ M_{1}\text{fl} = \frac{(1277.77)(30)}{8} \left( \frac{1}{1000} \right) \left( \frac{12}{1} \right) = 1724.99 \text{ k-in} \]

\[ M_{2}\text{fl} = \frac{(1093.37)(30)}{8} \left( \frac{1}{1000} \right) \left( \frac{12}{1} \right) = 1476.59 \text{ k-in} \]

**Total Moments**

1st Floor:

\[ M_{\text{rneeded}} + M_{\text{vibind}} = 379511 + 1724.99 = 39676.1 \text{ k-in} \]

2nd Floor:

\[ M_{\text{rneeded}} + M_{\text{vibind}} = 379511 + 1476.59 = 39427.79 \text{ k-in} \]

\[ Z_{x} = \frac{5.5116.07}{0.94(50)} = 122.67 \text{ in}^3 \]

\[ Z_{x2} = \frac{5.2715.59}{0.90(50)} = 117.15 \text{ in}^3 \]

1st Floor:

Use 421 x 55  \[ a = 126 \text{ in}^3 \]

\[ I_{x} = 1190 \text{ in}^4 \]

Add 55 kips to DL

\[ DL = 2.98 \times 145 + 55 = 353.14 \times 145 \times \frac{145}{2} \]

\[ 1.2(0.12) + 1.6LL = 1.2(3.53,14) + (426) = (423.77) + (426) = 843.77 \times \frac{145}{2} \]

\[ M_{0} = \frac{(1343.77)(30)}{8} \left( \frac{1}{1000} \right) \left( \frac{12}{1} \right) = 1814.09 \text{ k-in} \]

\[ M_{\text{tor}} = (1814.09) + (3.795.11) = 5609.09 \text{ k-in} \]

\[ Z_{x} = \frac{5.609.09}{0.90(50)} = 124.65 \text{ in}^3 \]

126 \geq 124.65 \text{ in}^3
Academic Building | Equation System

\[ \Delta T = \frac{40(12)}{2.40} = 1.5 \text{ in} \]

\[ L_2 = DL + 0.5L = (353.14) + 0.5(575) \]

\[ W_T = 690.0 \text{ kips} \]

\[ P = (212.76 + 0.5(1500)) \frac{23}{2} = 1747.6 \text{ kips} \]

\[ P = \frac{5(640.69)(30)}{384(2900)(110)} \left( \frac{1}{100} \right)^3 \]

\[ \Delta_{max} = 0.784 + 0.353 = 1.137 \text{ in} \leq 1.5 \text{ in} \]

\[ \Delta_L = \frac{40}{300} = \frac{30}{20} \frac{1.2}{50} = 1 \text{ in} \]

\[ W_L = 0.5LL = 0.5(575) = 287.5 \text{ kips} \]

\[ P = 0.5(1500) \frac{23}{2} = 8625 \text{ kips} \]

\[ d_{max} = \left( \frac{556.25}{25} \right) \left( \frac{3(30)^2 - 1(8.25)^2}{209(2900)(110)} \right) \left( \frac{1}{200} \right)^3 = 0.05761 + 0.159 = 0.535 \text{ in} \leq 1 \text{ in} \]

Use \( W_2 = 1.55 \) for vertical beam on 1st Floor

2nd Floor

Use \( W_2 = 1.55 \)

\[ Z_x = 1.26 \text{ in}^3 \quad I_x = 1.190 \text{ in}^4 \]

Add 55 \( \% \) to DL

\[ DL = 298.14 + 55 = 353.14 \text{ in} \]

\[ 12DL + 1.6LL = 1.2(353.14) + 736 = (425.77) + 736 = 1159.77 \text{ kips} \]

\[ M_x = \frac{(1159.77)(32)}{8} \left( \frac{1}{100} \right) \left( \frac{12}{1} \right) = 15.65 \text{ kips-in} \]

\[ M_{max} = 15.65 \text{ kips-in} + (3,745.15) = 5360.69 \text{ kips-in} \]

\[ Z_y = \frac{5360.69}{(0.9)(50)} = 119.13 \text{ in}^3 \]

\[ 12.6 = 119.13 \checkmark \]
\[\Delta t = \frac{1}{210} = \frac{96}{890} = 1.5 \text{ in}\]

\[W_p = 0.5 + 0.5 \left( \frac{353.19}{140} \right) = 0.5(160)\]
\[W_L = 583.19 \times 5 \text{ in}^2\]

\[P = (812.75) + 0.5(15000) = 22,171.25\]

\[\Delta_{\text{max}} = \left( \frac{17.71 \times 6.325}{24/50000} \right) \left( \frac{5 \times 383.19}{383.19 (29000) (1140)} \right) \left( \frac{1}{1000} \right) \left( \frac{12}{12} \right)\]

\[\Delta_{\text{max}} = 0.784 + 0.3218 = 1.105 \text{ in} \leq 1.5 \text{ in}\]

\[\Delta_{\text{LL}} = \frac{3200}{360} = 1 \text{ in}\]

\[W_{LL} = 0.5 LL = 0.5(480) = 240 \text{ lb}\]

\[p = 0.5(15000) \times \frac{5}{24} = 862.5 \text{ lb}\]

\[\Delta_{\text{max}} = \left( \frac{46.25 \times 8.25}{24/50000} \left( \frac{5(230)}{383.19 (29000) (1140)} \right) \left( \frac{1}{1000} \right) \left( \frac{12}{12} \right)\]

\[\Delta_{\text{max}} = 0.376 + 0.1265 = 0.502 \text{ in} \leq 1 \text{ in}\]

Use W21 x 55 for vertical beam on 2nd & 3rd floors
E = 29,000 ksi, F = 50 ksi.

Beam Length = 30'
2 equal-moment point loads

Concentrated Loads

\[ \omega_1 = 333.3 \text{ kips} \]

Beam = \[ \omega_1 \text{ kips} = \left(333.3 \text{ kips} \right) \left( \frac{25}{3} \right) = 38.932.95 \text{ kips} \]

\[ M_0 = 3.83 \text{ ft} \cdot \left(8.25\right) \left( \frac{1}{2} \right) = 37.75 \text{ kips-ft} \]

Distributed Load

Endzone = \[ 7.8\text{ in}^2 \times \text{height} = 15\text{ ft} = 570 \text{ lb/ft} \]

DL = \[ 7.2 \text{ (50)} = 684 \text{ lb/ft} \]

\[ M_0 = \left(684\right) \left(3.25\right)^2 \left( \frac{1}{1000} \right) \left( \frac{12}{T} \right) = 92.5 \text{ kip-ft} \]

\[ M_{\text{total}} = 37.75 + 92.5 = 130.25 \text{ kip-ft} \]

\[ Z_2 = \frac{4.718.4}{(0.9)(50)} = 104.85 \text{ in}^3 \Rightarrow 102.1 \times 50 \Rightarrow Z_2 = 110 \text{ in}^3 \]

\[ I_2 = 984 \text{ in}^4 \]

Add DL: 50 mi

DL = \[ 684 + 1.2 \text{ (50)} = 694 + 60 = 754 \text{ lb/ft} \]

\[ M_0 = \left(744\right) \left(3.25\right)^2 \left( \frac{1}{1000} \right) \left( \frac{12}{T} \right) = 1004.9 \text{ kip-ft} \]

\[ M_{\text{total}} = 37.75 + 1004.9 = 1042.64 \text{ kip-ft} \]

\[ Z_2 = \frac{4.729.7}{(0.9)(50)} = 106.65 \text{ in}^3 \leq 110 \text{ in}^3 \]
\[ \Delta T \leq \frac{7}{290} = 0.15 \text{ in} \]

\[ W_T = P_L = 570 + 50 = 620 \text{ lb/ft} \]

\[ \left[ \frac{3L^2 - 7A_2}{6A_2} \right] = 0.998 \]

\[ P = \left( \frac{12.75 + 0.5(1500)}{2} \right) = 17.97 \text{ lb/ft} \]

\[ \Delta_{max} = \left( 0.986 \right) + \left( \frac{5(620)(10^{-6})}{384(29000)(997)} \right) \left( \frac{1}{1000} \right) \left( \frac{17.97}{1} \right) \]

\[ \Delta_{max} = \left( 0.986 \right) + (0.364) = 1.350 \text{ in} \leq 1.5 \text{ in} \]

\[ \Delta_L \leq \frac{7}{260} = 0.11 \text{ in} \]

\[ p = 0.5 \left( 1500 \right) \left( \frac{23}{2} \right) = 862.5 \text{ in}^2 \]

\[ \Delta_{max} = 0.436 \text{ in} \leq 1 \text{ in} \]

\[ \text{Use W21x50} \]

\[ \Delta_L = 0.436 \]
Appendix H: Column Design

Appendix H.1: Column Excel Document Design Aid

<table>
<thead>
<tr>
<th>Column Bay Properties:</th>
<th>D</th>
<th>Units</th>
<th>Selected Section Properties:</th>
<th>Units</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tributary Width</td>
<td>43</td>
<td>ft</td>
<td>Column Height</td>
<td>15 ft</td>
</tr>
<tr>
<td>Tributary Length</td>
<td>32.5</td>
<td>ft</td>
<td>Section</td>
<td>W12x72</td>
</tr>
<tr>
<td>Total beam length enclosed</td>
<td>162.5</td>
<td>ft</td>
<td>Self Weight</td>
<td>72 lb/ft</td>
</tr>
<tr>
<td>Total girder length enclosed</td>
<td>43</td>
<td>ft</td>
<td>ry</td>
<td>3.04 in</td>
</tr>
<tr>
<td>Beam self weight</td>
<td>99</td>
<td>lb/ft</td>
<td>rx</td>
<td>5.32 in</td>
</tr>
<tr>
<td>Girder self weight</td>
<td>135</td>
<td>lb/ft</td>
<td>Fy</td>
<td>50 ksi</td>
</tr>
<tr>
<td>Total exterior length</td>
<td>0</td>
<td>ft</td>
<td>Es</td>
<td>29000 ksi</td>
</tr>
<tr>
<td></td>
<td></td>
<td></td>
<td>Ag</td>
<td>21.1 in^2</td>
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</tbody>
</table>

### Dead Loads:

<table>
<thead>
<tr>
<th></th>
<th>Units</th>
<th>Factored Pu w/ Self Weight</th>
<th>697.8246 kips</th>
</tr>
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<tbody>
<tr>
<td>MEP</td>
<td>8.85 psf</td>
<td></td>
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</tr>
<tr>
<td>Concrete Slab+Decking</td>
<td>40 psf</td>
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</tr>
<tr>
<td>Ceiling+Insulation</td>
<td>3 psf</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Exterior Enclosure</td>
<td>570 lb/ft</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Roof Dead Load</td>
<td>15 psf</td>
<td></td>
<td></td>
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<tr>
<td>TOTAL DEAD LOAD</td>
<td>248.12 kips</td>
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<td></td>
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</tbody>
</table>

### Live Loads:

<table>
<thead>
<tr>
<th></th>
<th>Units</th>
<th>Short Calculation</th>
<th>38.694 ksi</th>
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<tbody>
<tr>
<td>Service Load</td>
<td>80 psf</td>
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<td></td>
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<tr>
<td>TOTAL LIVE LOAD</td>
<td>223.6 kips</td>
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### Other Loads:

<table>
<thead>
<tr>
<th></th>
<th>Units</th>
<th>Long Calculation</th>
<th>71.598 ksi</th>
</tr>
</thead>
<tbody>
<tr>
<td>Snow</td>
<td>55 psf</td>
<td></td>
<td></td>
</tr>
<tr>
<td>TOTAL SNOW LOAD</td>
<td>76.86 kips</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

Total Factored Pu (for 2 floors): 693.94 kips

Consult Table 4-1a in AISC Manual
Appendix H.2: Column Design Hand Calculations

Tributary Width = 413 feet
Tributary Length = 32.5 feet
Total enclosed beam length = (5)(32.5) = 162.5 feet
Total enclosed Girder length = (1)(43) = 43 feet
Max enclosed beam self weight = 99 lb/ft
Max enclosed girder self weight = 135 lb/ft

Loads:
Dead Loads:
- MEP = 8.85 psf
- Concrete slab + decking = 40 psf
- Ceiling + Insulation = 3 psf
- Exterior Enclosure = 570 lb/ft
- Roof Dead Load = 15 psf

Live Loads:
- Service Load = 80 psf
- Other:
- Snow Load = 55 psf

* Design will be carried out for the 1st floor columns and the largest member size will be used in each story for ease of design.*
Dead Load Calculation:

\[ DL = \left[ (8.85 \text{psf} + 3 \text{psf}) \left( 43 \text{ft} \times 32.5 \text{ft} \right) \right] (3) \]
\[ + \left[ (162.5 \text{ft} \times 99 \text{lb/ft}) + (43 \text{ft} \times 135 \text{lb/ft}) \right] (3) \]
\[ + \left[ (40 \text{psf}) \left( 43 \text{ft} \times 32.5 \text{ft} \right) \right] (2) \]
\[ + \left[ (15 \text{psf}) \left( 43 \text{ft} \times 32.5 \text{ft} \right) \right] \]

\[ DL = 248120 \text{ lb} = 248.12 \text{ k} \]

Live Load Calculation:

\[ LL = (80 \text{psf}) \left( 43 \text{ft} \times 32.5 \text{ft} \right) (2) \]

\[ LL = 223600 \text{ lb} = 223.6 \text{ k} \]

Snow Load Calculation:

\[ S = (55 \text{psf}) \left( 43 \text{ft} \times 32.5 \text{ft} \right) \]

\[ S = 76862.5 \text{ lb} = 76.86 \text{ k} \]

Governing Load Combination: \( 1.2DL + 1.6LL + 0.5S \)

\[ P_u = 1.2(248.12k) + 1.6(223.6k) + 0.5(76.86k) \]

\[ P_u = 693.94k \]

Consult Table 4-1a in AISC Manual
Select: W12 x 72
Section Properties:
Self Weight = 72 lb/ft
FY = 3.04 in
Fy = 50 ksi
Es = 29,000 ksi
Ag = 21.1 in²
Φc Pn @ 15 ft = 735 k

New Pn including Self Weight = 699.4 k + 1.2(72 lb/ft)(15 ft)(3)
Pn = 697.82 k

Strength Check:
Φc Pn > Pn → 735 k > 697.82 k ✓

KL/r Check:
Assuming K = 1.0

Lx/rx = (15 ft)(12 in/ft)/(5.32 in) = 33.83
Ly/ry = (15 ft)(12 in/ft)/(3.04 in) = 59.21 → Governs

Long or Short Column?
4.71 \sqrt{E/Fy} = 4.71 \sqrt{29,000 ksi / 50 ksi} = 113.432

59.21 < 113.432 → Short to Intermediate Column.
\[ F_e = \frac{\pi^2 E}{(L/L)^2} = 81,639 \, \text{ksi} \]

\[ F_{cr} = (0.658 \left(\frac{F_y}{F_e}\right)) F_y = (0.658 \left(\frac{50\,\text{ksi}}{81.64\,\text{ksi}}\right)) 50\,\text{ksi} \]

\[ F_{cr} = 38.694 \, \text{ksi} \]

\[ P_{cr} = \phi F_{cr} A_y = 0.9 \times 38.694 \, \text{ksi} \times 21.1 \, \text{in}^2 \]

\[ P_{cr} = 734.8 \, \text{k} \]

\[ P_{cr} > P_u \implies 734.8 \, \text{k} > 634.22 \, \text{k} \checkmark \]

\[ \therefore \text{W12x72 is acceptable for use.} \]
Appendix I: Earthquake Load Excel Document Design Aid

<table>
<thead>
<tr>
<th>A</th>
<th>B</th>
<th>C</th>
<th>D</th>
<th>E</th>
<th>F</th>
<th>G</th>
<th>H</th>
<th>I</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Using Equivalent Lateral Force Procedure for Regular Multi-Level Building/Structural Systems</td>
<td></td>
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<tr>
<td></td>
<td>Per IBC 2012 and ASCE 7-10 Specifications</td>
<td></td>
<td></td>
<td></td>
<td></td>
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<td>Job Name:</td>
<td>Subject:</td>
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<td></td>
<td></td>
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<tr>
<td></td>
<td>Job Number:</td>
<td>Originator:</td>
<td>Checker:</td>
<td></td>
<td></td>
<td></td>
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</tr>
<tr>
<td>8</td>
<td>Risk Category =</td>
<td><strong>II</strong>: ASCE 7-10 Table 1604.5, page 336</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>9</td>
<td>Importance Factor, I =</td>
<td><strong>1.00</strong>: ASCE 7-10 Table 1.5-2, page 5</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>10</td>
<td>Soil Site Class =</td>
<td><strong>D</strong>: ASCE 7-10 Table 203-1, page 204</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>11</td>
<td>Location Zip Code =</td>
<td><strong>1609</strong></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>12</td>
<td>Spectral Accel., Ss =</td>
<td><strong>0.240</strong>: ASCE 7-10 Figures 22-1 to 22-11</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>13</td>
<td>Spectral Accel., St =</td>
<td><strong>0.077</strong>: ASCE 7-10 Figures 22-2 to 22-11</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>14</td>
<td>Long. Trans. Period, TL =</td>
<td><strong>6.000</strong>: sec. from ASCE 7-10 Fig's. 22-12 to 22-16</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>15</td>
<td>Structure Height, Hn =</td>
<td><strong>46.000</strong>: ft.</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
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<tr>
<td>16</td>
<td>Actual Calc. Period, Tc =</td>
<td><strong>0.000</strong>: sec. from independent analysis</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>17</td>
<td>Seismic Resist. System =</td>
<td><strong>B1</strong>: Steel eccentrically braced frames (ASCE 7-10 Table 12.2-1)</td>
<td></td>
<td></td>
<td></td>
<td></td>
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<td></td>
</tr>
</tbody>
</table>

**Structure Weight Distribution:**

<table>
<thead>
<tr>
<th>Seismic Level</th>
<th>Height, hx (ft)</th>
<th>Weight, Wx (kips)</th>
</tr>
</thead>
<tbody>
<tr>
<td>3</td>
<td>45.000</td>
<td>2675.45</td>
</tr>
<tr>
<td>2</td>
<td>30.000</td>
<td>3455.20</td>
</tr>
<tr>
<td>1</td>
<td>16.000</td>
<td>3600.20</td>
</tr>
</tbody>
</table>

**Results:**

Total Weight, W = \( \sum W_x = 9671.86 \) kips (ASCE 7-10 Section 12.7.2)

**Site Coefficients:**

\[
F_a = \frac{1.800}{2} \quad \text{ASCE 7-10 Table 11.4-1, page 66}
\]

\[
F_v = \frac{2.400}{2} \quad \text{ASCE Table 11.4-2, page 66}
\]

**Maximum Spectral Response Accelerations for Short and 1-Second Periods:**

\[
S_{sz} = 0.384 = S_{sz} = F_a S_z, \quad \text{ASCE Eqn. 11.4-1, page 85}
\]

\[
S_{sm} = 0.165 = S_{sm} = F_v S_m, \quad \text{ASCE Eqn. 11.4-2, page 65}
\]

**Design Spectral Response Accelerations for Short and 1-Second Periods:**

\[
S_{ds} = 0.256 = S_{ds} = 2S_{sz}/3, \quad \text{ASCE 7-10 Eqn. 11.4-3, page 65}
\]

\[
S_{dt} = 0.107 = S_{dt} = 2S_{sm}/3, \quad \text{ASCE 7-10 Eqn. 11.4-4, page 65}
\]

**Seismic Design Category:**

\[
\text{Category for Sz = } B; \quad \text{ASCE 7-10 Table 11.6-1, page 67}
\]

\[
\text{Category for Ss = } B; \quad \text{ASCE 7-10 Table 11.6-2, page 67}
\]

**Design Category =** B

Most critical of either category case above controls

**Fundamental Period:**

\[
V = C_a S_{sz} = 248.65 \text{ kips (Regular Bldg. Configurations Only)}
\]
### Maximum Spectral Response Accelerations for Short and 1-Second Periods:

<p>| | | |</p>
<table>
<thead>
<tr>
<th></th>
<th></th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>Sns = 0.384 Sns = Fe*Ss, ASCE Eqn. 11.4-1, page 65</td>
<td>Sm1 = 0.161 Sm1 = Fv*S1, ASCE Eqn. 11.4-2, page 65</td>
<td></td>
</tr>
</tbody>
</table>

### Design Spectral Response Accelerations for Short and 1-Second Periods:

<p>| | |</p>
<table>
<thead>
<tr>
<th></th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>Sds = 0.256 Sds = 2*Sns/3, ASCE 7-10 Eqn. 11.4-3, page 65</td>
<td></td>
</tr>
<tr>
<td>Sd1 = 0.107 Sd1 = 2*Sm1/3, ASCE Eqn. 11.4-4, page 65</td>
<td></td>
</tr>
</tbody>
</table>

### Seismic Design Category:

<table>
<thead>
<tr>
<th>Category for Sds</th>
<th>Category for Sd1</th>
</tr>
</thead>
<tbody>
<tr>
<td>B</td>
<td>B</td>
</tr>
</tbody>
</table>

**ASCE 7-10 Table 11.6-1, page 67**

**ASCE 7-10 Table 11.6-2, page 67**

Use Category = B

Most critical of either category case above controls

### Fundamental Period:

<table>
<thead>
<tr>
<th>Period Coefficient, Ct</th>
<th>0.030</th>
<th>ASCE 7-10 Table 12.8-2, page 90</th>
</tr>
</thead>
<tbody>
<tr>
<td>Period Exponent, x</td>
<td>0.75</td>
<td>ASCE 7-10 Table 12.8-2, page 90</td>
</tr>
<tr>
<td>Approx. Period, Ta</td>
<td>0.52</td>
<td>ASCE 7-10 Section 12.8.2.1, Eqn. 12.8-7</td>
</tr>
<tr>
<td>Upper LimitCoeff., Cu</td>
<td>1.686</td>
<td>ASCE 7-10 Table 12.8-1, page 90</td>
</tr>
<tr>
<td>Period max, T(max)</td>
<td>0.879</td>
<td>sec. T(max) = Cu*Ta, ASCE 7-10 Section 12.8.2, page 90</td>
</tr>
<tr>
<td>Fundamental Period, T</td>
<td>0.521</td>
<td>sec. T = Ta &lt;= Cu*Ta, ASCE 7-10 Section 12.8.2, page 90</td>
</tr>
</tbody>
</table>

### Seismic Design Coefficients and Factors:

<table>
<thead>
<tr>
<th>Response Mod. Coef., R</th>
<th>8</th>
<th>ASCE 7-10 Table 12.2.1, pages 73-75</th>
</tr>
</thead>
<tbody>
<tr>
<td>Ovrrstrong Factor, Cio</td>
<td>2</td>
<td>ASCE 7-10 Table 12.2.1, pages 73-75</td>
</tr>
<tr>
<td>Defl. Ampil. Factor, C4</td>
<td>4</td>
<td>ASCE 7-10 Table 12.2.1, pages 73-75</td>
</tr>
<tr>
<td>Cs</td>
<td>0.032</td>
<td>For T&lt;=TL, Cs(max) = SD1(T*(R/I)), ASCE 7-10 Eqn. 12.8-3</td>
</tr>
<tr>
<td>Cs(max)</td>
<td>0.026</td>
<td>For T&lt;=TL, Cs(max) = SD1(T*(R/I)), ASCE 7-10 Eqn. 12.8-3</td>
</tr>
<tr>
<td>Cs(min)</td>
<td>0.011</td>
<td>Cs(min) = 0.044*SDS'I &gt;= 0.01, ASCE 7-10 Eqn. 12.8-5</td>
</tr>
<tr>
<td>Use: Cs</td>
<td>0.026</td>
<td>Cs(min) &lt;= Cs &lt;= Cs(max)</td>
</tr>
</tbody>
</table>

### Seismic Base Shear:

| V = 248.65 kips, V = Cs*W, ASCE 7-10 Section 12.8.1, Eqn. 12.8-1 |

### Seismic Shear Vertical Distribution:

<table>
<thead>
<tr>
<th>Distribution Exponent, k = 1.01</th>
</tr>
</thead>
</table>

**k = 1** for **T<=0.5 sec.**, **k = 2** for **T>=2.5 sec.**

Lateral Force at Any Level: **Fv = Cvx*V**, ASCE 7-10 Section 12.8.3, Eqn. 12.8-11, page 91

Vertiical Distribution Factor: **Cvx = Wx*V/(SY*Wx)**, ASCE 7-10 Eqn. 12.8-12, page 91

<table>
<thead>
<tr>
<th>Seismic Level x</th>
<th>Weight, Wx (kips)</th>
<th>hw% (ft)</th>
<th>Wx*h%/ (ft-kips)</th>
<th>Cvx (%)</th>
<th>Shear, Fx (kips)</th>
<th>Σ Story Shears</th>
</tr>
</thead>
<tbody>
<tr>
<td>3</td>
<td>2676.45</td>
<td>46.856</td>
<td>125497.0</td>
<td>0.435</td>
<td>108.21</td>
<td>108.21</td>
</tr>
<tr>
<td>2</td>
<td>3495.20</td>
<td>31.103</td>
<td>108711.1</td>
<td>0.377</td>
<td>93.81</td>
<td>202.02</td>
</tr>
<tr>
<td>1</td>
<td>3500.20</td>
<td>15.437</td>
<td>54034.3</td>
<td>0.168</td>
<td>46.63</td>
<td>248.65</td>
</tr>
</tbody>
</table>

**Σ = 9671.95**

296152.4 1.000 248.65
Appendix J: Wind Load Excel Document Design Aid

<table>
<thead>
<tr>
<th>MWFRS Wind Loads</th>
<th>Job No: 11054</th>
</tr>
</thead>
<tbody>
<tr>
<td>ASCE7-10</td>
<td>Designer: DCB</td>
</tr>
<tr>
<td>Enclosed &amp; Partially Enclosed Buildings of All Heights</td>
<td>Checker:</td>
</tr>
<tr>
<td>Notes: Grinding Building (+/- Z Direction)</td>
<td>Date: 2/13/2019</td>
</tr>
</tbody>
</table>

### Basic Parameters
- Risk Category: II
- Basic Wind Speed, V: 120 mph
- Wind Directionality Factor, Kd: 0.85
- Exposure Category: C
- Topographic Factor, Kn: 1.00
- Gust Effect Factor, Go or Gd: 0.850
- Enclosure Classification: Enclosed
- Internal Pressure Coefficient, Gs: +/- 0.18
- Terrain Exposure Constant, α: 9.5
- Terrain Exposure Constant, z: 900 ft

### Wall Pressure Coefficients
- Windward Wall Width, B: 100 ft
- Side Wall Width, L: 250 ft
- L/B Ratio: 2.50
- Windward Wall Coefficient, Cw: 0.80
- Leeward Wall Coefficient, Cl: -0.28
- Side Wall Coefficient, Cs: -0.70

### Roof Pressure Coefficients
- Roof Slope, θ: 0.8°
- Median Roof Height, h: 45 ft
- Velocity Pressure Exposure Coef, Kp: 1.07
- Velocity Pressure, qa: 33.5 psf
- h/L Ratio: 0.18
- Windward Roof Area: 0 ft²
- Roof Area Within 23 ft of WW Edge: 0 ft²

### Structure Pressure Summary [Add Internal Pressure ± Gs or ± Gs, as Necessary]

<table>
<thead>
<tr>
<th>Height, z</th>
<th>Kd</th>
<th>qa</th>
<th>Kd, qa</th>
<th>WW</th>
<th>LW</th>
<th>WW + LW</th>
<th>Side</th>
<th>WW</th>
<th>LW</th>
<th>WW + LW</th>
</tr>
</thead>
<tbody>
<tr>
<td>0 ft</td>
<td>0.85</td>
<td>20.6 psf</td>
<td>18.1 psf</td>
<td>25.9 psf</td>
<td>-25.6 psf</td>
<td>-19.9 psf</td>
<td>Min:</td>
<td>-25.6 psf</td>
<td>Min:</td>
<td>6.0 psf</td>
</tr>
<tr>
<td>5 ft</td>
<td>0.85</td>
<td>20.6 psf</td>
<td>18.1 psf</td>
<td>25.9 psf</td>
<td>-25.6 psf</td>
<td>-19.9 psf</td>
<td>Min:</td>
<td>-25.6 psf</td>
<td>Min:</td>
<td>6.0 psf</td>
</tr>
<tr>
<td>9 ft</td>
<td>0.85</td>
<td>20.6 psf</td>
<td>18.1 psf</td>
<td>25.9 psf</td>
<td>-25.6 psf</td>
<td>-19.9 psf</td>
<td>Min:</td>
<td>-25.6 psf</td>
<td>Min:</td>
<td>6.0 psf</td>
</tr>
<tr>
<td>14 ft</td>
<td>0.85</td>
<td>26.6 psf</td>
<td>18.1 psf</td>
<td>25.9 psf</td>
<td>-25.6 psf</td>
<td>-19.9 psf</td>
<td>Min:</td>
<td>-25.6 psf</td>
<td>Min:</td>
<td>6.0 psf</td>
</tr>
<tr>
<td>18 ft</td>
<td>0.88</td>
<td>27.6 psf</td>
<td>18.8 psf</td>
<td>25.9 psf</td>
<td>-25.6 psf</td>
<td>-19.9 psf</td>
<td>Min:</td>
<td>-25.6 psf</td>
<td>Min:</td>
<td>6.0 psf</td>
</tr>
<tr>
<td>23 ft</td>
<td>0.92</td>
<td>29.6 psf</td>
<td>19.7 psf</td>
<td>25.9 psf</td>
<td>-25.6 psf</td>
<td>-19.9 psf</td>
<td>Min:</td>
<td>-25.6 psf</td>
<td>Min:</td>
<td>6.0 psf</td>
</tr>
<tr>
<td>27 ft</td>
<td>0.96</td>
<td>30.1 psf</td>
<td>20.5 psf</td>
<td>25.9 psf</td>
<td>-25.6 psf</td>
<td>-19.9 psf</td>
<td>Min:</td>
<td>-25.6 psf</td>
<td>Min:</td>
<td>6.0 psf</td>
</tr>
<tr>
<td>32 ft</td>
<td>0.99</td>
<td>31.1 psf</td>
<td>21.9 psf</td>
<td>25.9 psf</td>
<td>-25.6 psf</td>
<td>-19.9 psf</td>
<td>Min:</td>
<td>-25.6 psf</td>
<td>Min:</td>
<td>6.0 psf</td>
</tr>
<tr>
<td>36 ft</td>
<td>1.02</td>
<td>32.0 psf</td>
<td>22.7 psf</td>
<td>25.9 psf</td>
<td>-25.6 psf</td>
<td>-19.9 psf</td>
<td>Min:</td>
<td>-25.6 psf</td>
<td>Min:</td>
<td>6.0 psf</td>
</tr>
<tr>
<td>41 ft</td>
<td>1.05</td>
<td>32.8 psf</td>
<td>22.3 psf</td>
<td>25.9 psf</td>
<td>-25.6 psf</td>
<td>-19.9 psf</td>
<td>Min:</td>
<td>-25.6 psf</td>
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<td>6.0 psf</td>
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<tr>
<td>45 ft</td>
<td>1.07</td>
<td>33.5 psf</td>
<td>22.8 psf</td>
<td>25.9 psf</td>
<td>-25.6 psf</td>
<td>-19.9 psf</td>
<td>Min:</td>
<td>-25.6 psf</td>
<td>Min:</td>
<td>6.0 psf</td>
</tr>
</tbody>
</table>
Appendix K: Building Story Drift Excel Document Design Aid
Appendix L: RISA 2D Building Models for Lateral Reinforcement Design

```
<table>
<thead>
<tr>
<th>Joint Label</th>
<th>X [in]</th>
<th>Y [in]</th>
<th>Rotation</th>
</tr>
</thead>
<tbody>
<tr>
<td>N1</td>
<td>0</td>
<td>0</td>
<td>0</td>
</tr>
<tr>
<td>N2</td>
<td>0</td>
<td>0</td>
<td>0</td>
</tr>
<tr>
<td>N3</td>
<td>-0.885</td>
<td>-0.69</td>
<td>3.282e-03</td>
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<tr>
<td>N4</td>
<td>-1.692</td>
<td>-1.31</td>
<td>1.366e-03</td>
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<td>N5</td>
<td>-2.184</td>
<td>-1.33</td>
<td>2.039e-03</td>
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<td>N6</td>
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<td>0.086</td>
<td>5.34e-03</td>
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<tr>
<td>N7</td>
<td>-1.819</td>
<td>0.99</td>
<td>3.762e-03</td>
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<tr>
<td>N8</td>
<td>-2.19</td>
<td>0.117</td>
<td>1.491e-03</td>
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</tbody>
</table>
```

Results for LC 4, 1.2D + E + L + 0.2S
Appendix M: Baseplate and Footing Design Hand Calculations

Base Plate Design

Column: \( W = 12 \times 7.2 \) \( A_c = 21.1 \text{ in}^2 \) \( d = 12.5 \text{ in} \) \( b = 12.0 \text{ in} \)

\( P_u = 697.8 \text{ kips} + 1.3 \text{ kips} + 65.7 \text{ kips} = 762.8 \text{ kips} \)

\( f_c = 9 \text{ ksi} \)

Use Area Forcing \( (A) \) greater than Area Base Plate \( (A_b) \)

\( P_u = \phi_f P_o \)

\( \phi_c = 0.65 \)

\( f_c = (0.85f_{cley} A_t) \) \( N/A_t \leq 2 \)

\( P_o = (0.65)(0.85)\left(A_1\right) \) \( (1) \)

\( 762.8 \text{ kips} = 7.62 \text{ kips} \)

\( A_t = 172.58 \text{ in}^2 \)

\( N = \Delta A + \Delta \)

\( \Delta = 0.5(0.85f_{cley} - 0.8f_{cley}) = 0.5(0.15(12.5) - 0.15(12)) \)

\( \Delta = 0.5(11.685 - 9.6) = 0.5(2.085) \approx 1.043 \text{ in} \)

\( N = 13.357 + 1.043 = 14.38 \text{ in} \) rounded to nearest even inch \( \Rightarrow 16 \text{ in} \)

\( B = \frac{A_1}{N} = \frac{172.58}{16} = 10.79 \text{ in} \) \( \Rightarrow \) \( B = 11 \text{ in} \)

\( A_t = \left(\frac{16}{12}\right) = 1.33 \text{ in}^2 \)

\( m = \frac{N - 0.95d}{2} = \frac{(16) - (11.685)}{2} = 2.1575 \text{ in} \)

\( n = B - 0.8b = \frac{(12) - (9.6)}{2} = 1.2 \text{ in} \)

\( n' = \frac{A_t}{f} = \frac{A_t}{f_{cley}} = \frac{1.33}{f_{cley}} = 3.037 \text{ in} \)

\( n' = m > n \)

\( t_{required} = \frac{2P_o}{f_{cley} \phi_c F_o N} \approx 3.057 \text{ in} \)

\( t_{required} = \frac{2(762.8)}{(0.65)(0.85)(10)(12)} = 1.278 \text{ in} \) \( \Rightarrow 1.5 \text{ in} \)

Base Plate is \( 16 \times 12 \times 1.5 \text{ in} \)
Minimum reinforcement connecting baseplate to footing

\[ A_s = 0.005A_1 \]

OSHA requires minimum 4 reinforcement anchors (1 in each corner)

\[ A_s = 0.005(12'' \times 16'') = 0.76 \text{ in}^2 \]

\[ 4A_b^2 = A_s \rightarrow 4A_b = 0.76 \text{ in}^2 \rightarrow A_b = 0.19 \text{ in}^2 \]

Use No 5 bolts \( (A = 0.31 \text{ in}^2) \)

Distance between center of holes \( \geq 3d = 3(\frac{1}{16}) = \frac{83}{16}'' \)

Minimum edge distance from center of hole \( = \frac{3}{8}'' \)

Place holes 1'' in from each edge (1 in each corner)

Thickness \( = 1.5'' \)
Concrete Footing

\[ f_c = 4140 \quad f_a = 3 \quad \gamma_{wet} = 145 \text{pcf} \quad \text{Assume 5' deep} \]

\[ q_e = 3000 - 5(145) = 3000 - 725 = 2275 \text{psf} \]

\[ A_{req} = \frac{D+L}{q_e} = \frac{593.75}{2.275} = 260.94 \text{ ft}^2 \times 5' = 1305 \text{ ft}^3 \times 145 \text{pcf} \]

Self-weight = 1817 kips

\[ A_{req} = \frac{593.75 + 1817}{2.275} = 394.16 \text{ ft}^2 \times 5' = 1720.8 \times 145 = 250 \text{ kips} \]

\[ A_{req} = \frac{593.75 + 250}{2.275} = 370.67 \times 5' = 1853 \times 145 = 269 \text{ kips} \]

\[ A_{req} = \frac{593.75 + 269}{2.275} = 379.11 \times 5' = 1895.6 \times 145 = 275 \text{ kips} \]

\[ A_{req} = \frac{593.75 + 275}{2.275} = 381.81 \times 5' = 1907.02 \times 145 = 277 \text{ kips} \]

\[ A_{req} = \frac{593.75 + 277}{2.275} = 382.75 \text{ ft}^2 \]

Use 20' x 20' square footings

\[ q_{o} = \frac{762.8 + 1.2(277)}{(20')}^2 = \frac{762.8 + 332.4}{400} = \frac{2174}{400} = 2.74 \text{ kips} \]
Concrete Footing

Punching Shear \( \phi V_c = \frac{0.75 \cdot 0.85}{2} \cdot \frac{1400}{1000} \cdot \frac{56 + 4d}{19.4} \cdot d = 10.63d + 0.76d^2 \)

\( V_c = 649.81 \text{ kips} \)

\( \phi V_c = 0.75 \cdot 0.85 \cdot \frac{1400}{1000} \cdot (20 \times 16) \cdot (28.51) \)

\( \phi V_c = 649.81 \text{ kips} \)

\( V_c = 9 \left( \frac{20 - \frac{13}{2}}{2} \right) \left( \frac{28.51}{12} \right) \cdot (20) = 2.74 \left( 9.5 - 2.375 \right) \cdot (20) \)

\( V_u = 390.267 \text{ kips} \)

\( \phi V_c > V_u \) \( 649.81 > 390.267 \)

\( \phi P_1 = 0.65 \cdot (2)(0.85) \cdot (4 \text{ ksi}) \cdot (12'' \times 16'') = 848.64 \text{ kips} \)

\( \phi P_1 > 762.8 \text{ kips} \)
Pulls:

\[ M_u = (q \cdot b) \frac{d^2}{2} \]

\[ l = \frac{b-a}{2} = 9.5' \]

\[ M_u = (2.74)(20)(9.5^2) = 2472.35 \text{ ft-lb} \times 12 = 29674.2 \text{ k-in} \]

\[ M_u = \phi \rho f_y bd^2 \left( 1 - 0.54 \frac{f_y}{f_y} \right) \]

\[ A_s = \rho bd \]

\[ 29674.2 = (0.9) \rho (60)/(2.0)^2 \left( 28.5^2 \right) \left( 1 - 0.54 \rho (60)/(20) \right) \]

\[ 0 = -29674.2 + 19566.32985 \rho - 93123516.39 \rho^2 \]

\[ \rho = 0.1101 \]

\[ A_s = 0.1101 (20 \cdot 12)(28.5^2) = 754.209 \text{ in}^2 \]

\#18 bars have 4 in² area

\[ \frac{754.209}{4} = 190 \#18 \text{ bars (95 in each direction)} \]

Cover = 2.5"  
\#18 bars = 2.257 in² x 95 bars = 214.975 in

\[ 240' - 2.5 = 235" \]

1.5d₃ spacing between bars = 3.386"

\[ d₃ = 2.257 (b) + (3.386b - 1) \]

\[ d₃ = 5.6925b - 3.866 \Rightarrow 238.386 = 5.6925b \]

\[ 12.25b \Rightarrow 12 \text{ bars/layer} \]

Use 32 bars/layer with 3 layers in each direction (alternating)

layers 1" above lower layer

5.25" spacing between bars
Concrete footing

- 20' x 12' Center
- 12' x 12' Base Plate
- 5' x 5'1" Top
- 28.5" x 5' Edge
- 18" x 2.25" Tie 0.5" x 2.125"
### Appendix N: Cost Estimate of Academic Structural Frame

<table>
<thead>
<tr>
<th>Item Type</th>
<th>Specific Item</th>
<th>Cost per Unit</th>
<th>Cost per Unit+OP</th>
<th>Units</th>
<th>Units/Roof</th>
<th>Units/2nd</th>
<th>Units/1st</th>
<th>Total Cost</th>
<th>Total Cost+OP</th>
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<tbody>
<tr>
<td>Beam</td>
<td>W14x22</td>
<td>42.86</td>
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<td>Concrete Slab</td>
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<td>Concrete Footing Over 5' CY, direct</td>
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<td>26 S/CY</td>
<td>74.07 per column</td>
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<td>Reinforcement #18 Bars</td>
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<td>2303.64</td>
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MQP LDA-1904
Appendix O: Cost Estimate of Academic Non-Structural Elements

<table>
<thead>
<tr>
<th>Item Type</th>
<th>Specific Item</th>
<th>Cost per Unit</th>
<th>Cost per Unit+OP</th>
<th>Units</th>
<th>Number of Units</th>
<th>Total Cost</th>
<th>Total Cost+OP</th>
</tr>
</thead>
<tbody>
<tr>
<td>Demolition</td>
<td>Small/single bldg, no salvage - wood</td>
<td>0.31</td>
<td>0.39</td>
<td>$/CF</td>
<td>2252500</td>
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<td>Exterior Enclosure</td>
<td>Brick Veneer (on rigid steel)</td>
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<td>162</td>
<td>$/SF(floor)</td>
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<td>Interior</td>
<td>Interior partitions</td>
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<td>Interior</td>
<td>Wall Finishes</td>
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<td>Floor Finishes</td>
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<td>$/SF(ceiling)</td>
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